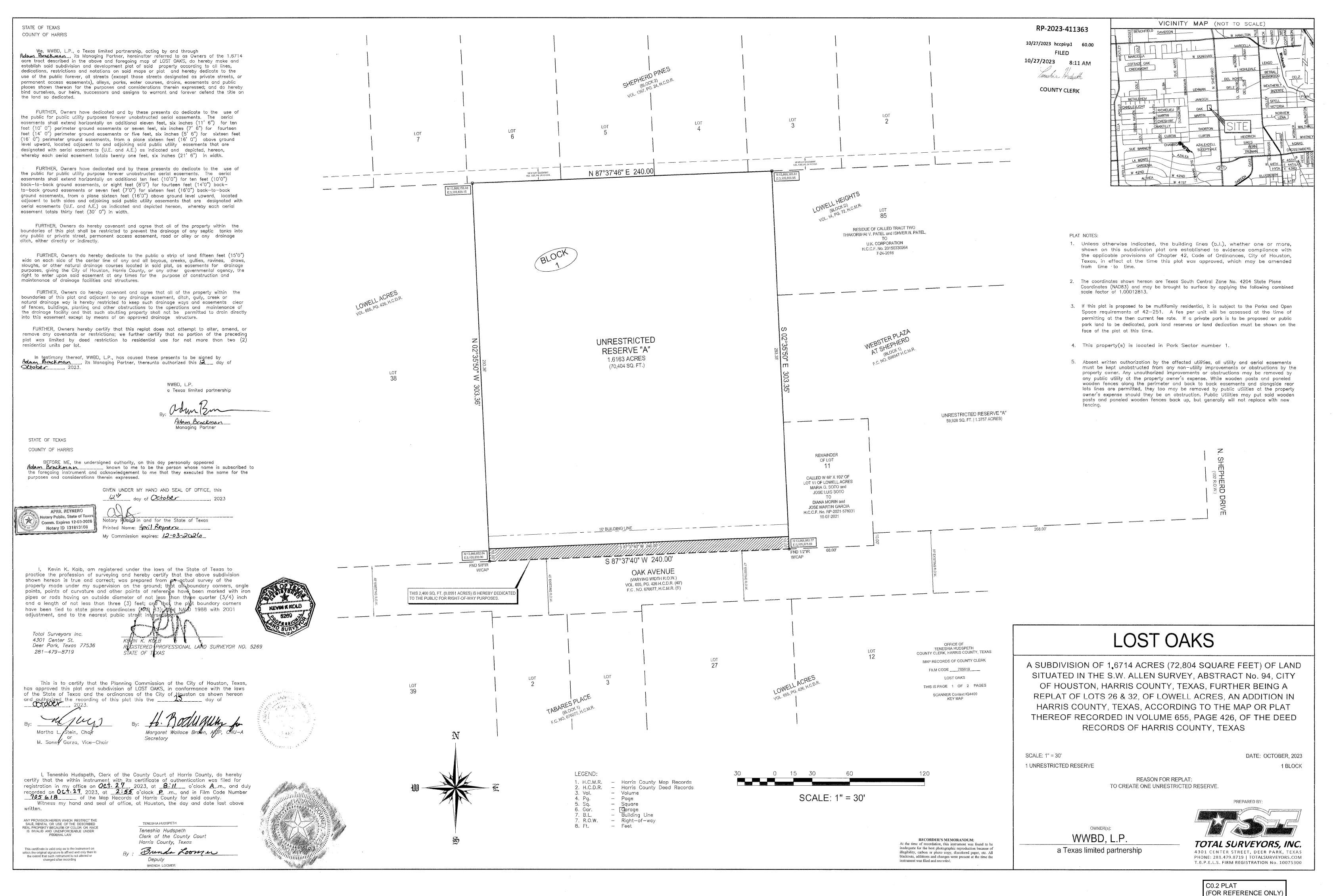


(FOR REFERENCE ONLY)



#### **GENERAL CONSTRUCTION NOTES**

- 1. ALL PROPOSED CONSTRUCTION SHALL CONFORM TO THE LATEST STANDARDS. CODES AND SPECIFICATION OF CITY OF HOUSTON AND HARRIS COUNTY. WHERE CONFLICT EXIST THE MORE STRINGENT REQUIREMENT SHALL BE ENFORCED.
- 2. CONTRACTOR SHALL LAYOUT THE ENTIRE PROJECT AND VERIFY CRITICAL DIMENSIONS, LOCATIONS, ALL EXISTING BENCHMARKS AND ELEVATIONS PRIOR TO THE START OF CONSTRUCTION. BRING ANY DISCREPANCIES TO THE ENGINEER FOR SOLUTION PRIOR TO PROCEEDING WITH THE WORK. IF NO VERIFICATION IS CONDUCTED CONTRACTOR SHALL ASSUME ALL RESPONSIBILITY
- 3. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGE TO THE EXISTING PUBLIC OR PRIVATE UTILITY LINES, INCLUDING BUT NOT LIMITED TO WATER LINES, WASTEWATER COLLECTION SYSTEMS AND STORM SEWERS, DURING CONSTRUCTION, ALL DAMAGES SHALL BE REPAIRED IN ACCORDANCE WITH CITY OF HOUSTON, DEPARTMENT OF PUBLIC WORKS AND ENGINEERING "STANDARD CONSTRUCTION SPECIFICATIONS" WITH LATEST ADDENDA AND AMENDMENTS THERETO, WITH NO COST TO THE PUBLIC.
- 4. EXISTING UTILITY INFORMATION SHOWN IS NOT GUARANTEED TO BE ACCURATE AND ALL INCLUSIVE. ALL EXISTING UTILITY LOCATIONS ARE APPROXIMATE. CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING UTILITIES AND OTHER FACILITIES PRIOR TO CONSTRUCTION. ANY CONFLICT OR DISCREPANCY DISCOVERED MUST BE IMMEDIATELY BROUGHT TO THE ENGINEER'S ATTENTION.
- 5. THE CONTRACTOR SHALL NOTIFY THE CITY OF HOUSTON, ENGINEERING DEPARTMENT HARRIS COUNTY ENGINEERING DEPARTMENT, CENTERPOINT ENERGY AND ALL OTHER APPLICABLE AGENCIES AT LEAST 48 HOURS PRIOR TO COMMENCEMENT OF WORK, NOTIFICATIONS SHALL BE FOLLOWED WITH A LETTER, COPIES OF WHICH SHALL BE SENT TO THE ENGINEER.
- 6. THE CONTRACTOR, ON BEHALF OF THE OWNER, IS TO OBTAIN ALL NECESSARY PERMITS REQUIRED BY THE CITY OF HOUSTON OR HARRIS COUNTY. PRIOR TO STARTING CONSTRUCTION.
- 7. GUIDELINES SET FORTH IN THE TEXAS "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", AS CURRENTLY AMENDED, SHALL BE OBSERVED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE FLAG MEN, SIGNAGE, STRIPING, AND WARNING DEVICES, ETC... DURING CONSTRUCTION - BOTH DAY AND NIGHT.
- 8 THE LOADING AND UNLOADING OF ALL PIPE VALVES HYDRANTS MANHOLES AND OTHER ACCESSORIES SHALL BE IN ACCORDANCE WITH THE MANUFACTURER RECOMMENDED PRACTICES AND SHALL AT ALL TIMES BE PERFORMED WITH CARE TO AVOID ANY DAMAGE TO THE MATERIAL. THE CONTRACTOR SHALL LOCATE AND PROVIDE THE NECESSARY STORAGE AREAS FOR THE MATERIALS AND EQUIPMENT.
- 9 THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SHIPPING AND STORING OF ALL MATERIALS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO EXAMINE SUCH MATERIAL AT THE POINT OF DELIVERY AND TO REJECT ALL DEFECTIVE MATERIAL. ANY DEFECTIVE MATERIAL INCORPORATED INTO THE WORK SHALL BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE. THERE SHALL BE NO PAYMENT MADE FOR STORED MATERIAL
- 10. ALL PIPE AND REINFORCEMENT STEEL SHALL BE KEPT FREE OF DIRT AND OTHER DEBRIS. ANY DAMAGE TO THE COATING OF THE VARIOUS MATERIALS MUST BE REPAIRED.
- 11. ADEQUATE DRAINAGE SHALL BE MAINTAINED AT ALL TIMES DURING CONSTRUCTION AND ANY DRAINAGE DITCH OR STRUCTURE DISTURBED DURING CONSTRUCTION SHALL BE RESTORED TO THE SATISFACTION OF THE OWNING AUTHORITY ALL CONSTRUCTION STORM RUNOFF SHALL COMPLY WITH THE FINAL DRAFT OF STORMWATER MANAGEMENT HANDBOOK FOR CONSTRUCTION ACTIVITIES AS PREPARED BY HARRIS COUNTY/HCFCD, AND THE CITY OF HOUSTON ALL IN COMLIANCE WITH THE TEXAS POLLUTANT DISCHARGE ELIMINATION SYSTEM (TPDES)
- 12. ACCESS TO ALL EXISTING STREETS AND DRIVEWAYS SHALL BE MAINTAINED AT ALL TIMES. 13. CONTRACTOR IS TO MAINTAIN A CLEAN PROJECT AREA, FREE FROM WORKMAN TRASH AND REFUSE. AT ALL TIMES.
- 14. THE CONTRACTOR IS REQUIRED TO FOLLOW ALL APPLICABLE OSHA RULES AND REGULATIONS TRENCH SAFETY SHALL BE DONE IN ACCORDANCE WITH OSHA STANDARDS.
- 15. SITE CONTRACTOR SHALL COORDINATE THE LOCATION OF ALL UTILITIES WITH THE BUILDING
- 16. ALL GEOTECHNICAL REPORTS (IF ANY) FOR THIS PROJECT ARE AVAILABLE FOR REFERENCE AT THE OFFICE OF THE ENGINEER.
- 17. SURFACE RESTORATION: AT THE END OF ALL CONSTRUCTION PROJECTS. THE CONTRACTOR SHALL RESTORE THE EXISTING FACILITIES LE THE PROPERTY FOLIAL TO OR BETTER THAN EXISTING SITE CONDITIONS PRIOR TO THE CONSTRUCTION. ALL FINISHED GRADES SHALL VARY

#### WATERLINE CONSTRUCTION NOTES

UNIFORMLY BETWEEN THE FINISHED ELEVATIONS SHOWN.

CONTRACTOR.

PIPE MOVEMENT

SEWER MANHOLE AND WATERLINE.

- 1 WATER LINES SHALL BE CONSTRUCTED IN ACCORDANCE WITH CITY OF HOUSTON, HOUSTON PUBLIC WORKS STANDARD SPECIFICATIONS AND STANDARD CONSTRUCTION DETAILS FOR WASTEWATER COLLECTION SYSTEMS. WATER LINES. STORM DRAINAGE AND STREET PAVING 2 4" THRU 12" WATER LINES SHALL BE P.V.C. CLASS 150, DR-18, AWWA C-900, SMALLER THAN 4" TER LINE SHALL BE PVC SCH 40 AS PER ASTM D1785 OR SEAMLESS COPPER TUBING TYPE "K" AS PER ASTM B88.
- 3. ALL WATERLINES, AFTER INSTALLATION, SHALL BE THOROUGHLY DISINFECTED ACCORDING TO AWWA SPECIFICATION C-651 AND THEN FLUSHED BEFORE BEING PLACED INTO SERVICE. AT LEAST ONE WATER SAMPLE PER 1000 FEET OF FINISHED LINE MUST BE COLLECTED AND SUBMITTED FOR BACTERIOLOGICAL ANALYSIS TO A LABORATORY CERTIFIED BY THE TEXAS DEPARTMENT OF HEALTH AND MUST MEET DEPARTMENT OF HEALTH AND CITY OF HOUSTON REQUIREMENTS PRIOR TO PLACING LINES INTO SERVICE.
- 4. HYDROSTATIC TESTING: ALL WATER PIPE SHALL BE TESTED FOR LEAKAGE IN ACCORDANCE WITH AWWA C-600, SECTION STANDARDS FOR A DURATION OF NOT LESS THAN TWO HOURS. LEAKAGE SHALL BE DEFINED AS THE QUANTITY OF WATER THAT MUST BE SUPPLIED INTO THE NEWLY LAID PIPE OR ANY VALVED SECTION THEREOF. TO MAINTAIN PRESSURE WITHIN 5 PSI OF THE SPECIFIED TEST PRESSURE AFTER THE PIPE HAS BEEN FILLED WITH WATER AND THE AIR HAS BEEN EXPELLED. THE TEST PRESSURE SHALL BE EITHER A MINIMUM OF 125 PSIG OR 1.5 TIMES THE MAXIMUM DESIGN PRESSURE WHICHEVER IS LARGER. THE MAXIMUM LEAKAGE. SHALL BE CALCULATED USING THE FORMULA AS FOLLOWS:

 $L = (S)(D)(P^1/2)/133,200$ WHERE L = ALLOWABLE LEAKAGE IN GAL./HR. S = LENGTH OF PIPE TESTED IN FEET D = INSIDE DIAMETER OF PIPE IN INCHES P = PRESSURE IN POUNDS PER SQUARE INCH (GAUGE)

- 5. 2,500 PSI CONCRETE THRUST BLOCKS SHALL BE PROVIDED AT ALL UNDERGROUND TEES BENDS AND LATERALS. THEY SHALL BE BUILT AS PER THE DETAILS PROVIDED TO PREVENT
- 6. ALL ABOVE GROUND DUCTILE IRON PIPE CONNECTIONS SHALL BE FLANGED. UNDERGROUND DUCTILE IRON PIPE CONNECTIONS SHALL BE BOLTLESS AND PUSH-ON AFTER THE FIRST
- FLANGED FITTING BELOW GRADE UNLESS NOTED OTHERWISE ON THE PLANS.
- 7. ALL FLANGES BELOW GRADE SHALL HAVE STAINLESS STEEL BOLTS AND NUTS AND SHALL BE INSULATED.
- 8 ALL WATER VALVES SHALL OPEN COUNTER CLOCKWISE, ALL WATER VALVES SHALL BE SUPPLIED AND INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF AWWA C-500 AND
- 9. ALL BELOW GRADE VALVES SHALL BE GASKETED, HUB-END GATE VALVES WITH A CAST IRON
- BOX, EXCEPT WHERE FLANGES ARE CALLED OUT ON THE PLANS 10 ALL WATER LINES TO HAVE 4' MINIMUM COVER TO FINISHED GRADE AND MINIMUM 12"

11A. MAINTAIN MINIMUM 9-F00T HORIZONTAL CLEARANCE BETWEEN OUTSIDE OF SANITARY

- CLEARANCE TO OTHER UTILITIES AT CROSSING UNLESS OTHERWISE NOTED ON PLANS. ALL WATER LINE INSTALLED OVER 8' DEEP SHALL UTILIZE RESTRAINED JOINT FITTINGS
- 11B. WATER LINES PARALLEL TO SANITARY LINES SHALL BE INSTALLED WITH AT LEAST A 9-FOOT
- CLEARANCE AND IN SEPARATE TRENCHES. 12. SANITARY PRECAUTIONS MUST BE TAKEN DURING WATERLINE CONSTRUCTION, PER AWWA

STANDARDS. PRECAUTIONS INCLUDE KEEPING THE PIPE CLEAN AND CAPPING OR OTHERWISE

SOURCES OF CONTAMINATION FORM UNFINISHED PIPE LINES AT TIMES WHEN CONSTRUCTION IS NOT IN PROGRESS. 13. CONTRACTOR SHALL PROVIDE FOR A MINIMUM OF 2 FEET CLEARANCE AT THE STORM SEWER AND WATER LINE CROSSINGS AND SANITARY SEWER AND WATER LINE CROSSINGS. THE

FEFECTIVELY COVERING OPEN PIPE ENDS TO EXCLUDE INSECTS. ANIMALS OR OTHER

- WATER LINE SHALL BE LOCATED AT A HIGHER LEVEL THAN THE SANITARY SEWER WHENEVER POSSIBLE.
- 14. ALL WATER LINE FITTINGS SHALL BE DUCTILE IRON COMPACT FITTINGS PER AWWA 153, UNLESS OTHERWISE NOTED.
- 15. THE CENTER OF FIRE HYDRANTS ARE TO BE LOCATED 3'-0" BEHIND THE BACK OF CURBS UNLESS OTHERWISE SHOWN. THE STEAMER NOZZLE SHALL BE A MINIMUM OF 18 (AND A MAXIMUM OF 30) INCHES ABOVE FINISHED GRADE. AND SHALL FACE THE STREET PAVEMENT UNLESS OTHERWISE SHOWN. ALL FIRE HYDRANTS SHALL BE PAINTED IN ACCORDANCE WITH THE CITY OF HOUSTON STANDARDS.

#### SANITARY SEWER CONSTRUCTION NOTES

1. ALL SEWERS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE CITY OF HOUSTON "STANDARD CONSTRUCTION SPECIFICATIONS FOR WASTEWATER COLLECTION SYSTEMS, WATER LINES, STORM DRAINAGE, STREET PAVING, AND TRAFFIC" AND ALL CURRENT AMENDMENTS THERETO AND BE SUBJECT TO A STANDARD EXFILTRATION TEST. TESTS ARE TO BE PERFORMED ON THE TOTAL FOOTAGE OF SEWER LINE INCLUDED IN THE PROJECT. REQUIREMENTS OF TEXAS ADMINISTRATIVE CODE, TITLE 30 CHAPTER 317, "DESIGN CRITERIA FOR SEWAGE SYSTEMS" SHALL

GOVERN WHERE CONFLICTS EXIST EXCEPT WHERE CITY REQUIREMENTS ARE MORE STRINGENT.

- 2. ALL MANHOLES ARE TO BE PER CITY OF HOUSTON STANDARD DETAILS DRAWING NUMBERS 02082-01, 02082-02, 02082N-02, 02082-03, AND 02082N-03 UNLESS OTHERWISE NOTED. USE 2010
- 3. SANITARY SEWER MANHOLES WILL HAVE BEDDING AND BACKFILL PER CITY OF HOUSTON STANDARD DETAILS DRAWING NO. 02317-08 UNLESS OTHERWISE NOTED.
- 4. THE SANITARY SEWER PVC PIPE SHALL BE ASTM D 3034 TYPE PSM SDR 26 GRAVITY SEWER PIPE. ASTM D2241 SDR 26 PRESSURE RATED SEWER PIPE OR AWWA C-900 DR-18 GREEN PVC PRESSURE RATED SEWER PIPE BASED ON CONSTRUCTION CONDITION REQUIREMENT AND CONFORMING TO ASTM D1784 AND CITY OF HOUSTON STANDARD SPECIFICATION SECTION 02506 POLYVINYL CHLORIDE PIPE.
- 5. WHEN SS PRESSURE RATED PVC PIPE IS USED ON WATERLINE (WL) CROSSING UNDER CONDITION 1 OF COH IDM TABLE 7.3, THE SAME TYPE OF D2241 SDR 26 PVC PIPE OR C-900 GREEN DR-18 PVC
  7. NECESSARY TESTING OF SUBGRADE AND PAVEMENT TO PROVE THAT THESE ITEMS MEET GREEN PRESSURED TO BE UTILIZING IN-BETWEEN TWO SS MH'S. OR TO UTILIZE A DI TRANSITION ADAPTER FOR THE CONNECTING OF ASTM D-3034 PVC GRAVITY PIPE TO DI-OD AWWA C-900 PVC PIPE CENTERED AT WL WHEN CONNECTING TWO DIFFERENT TYPES OF PVC PIPES FOR SEWER CONSTRUCTION.
- 6. AWWA C-900 DR-18 PVC PIPE USES EITHER AWWA C900 DR-18 PVC FITTINGS OR DIP FITTINGS. 7. ALL SANITARY SEWER LINES UNDER PROPOSED OR FUTURE PAVEMENT AND TO A POINT ONE (1) FOOT BACK OF ALL PROPOSED OR FUTURE CURBS SHALL HAVE BEDDING PER CITY OF HOUSTON STANDARD DETAILS DRAWING NUMBERS 02317-01, 02317-02, OR 02317-03 AS APPLICABLE, WITH 1 ½ SACK CEMENT/CY STABILIZED SAND BACKFILL UP TO THE BOTTOM OF THE PAVEMENT SUBGRADE. 100 PSI PERFORMANCE RESULTS ARE STILL REQUIRED
- 8. ALL SANITARY SEWERS CROSSING WATER LINES WITH A CLEARANCE BETWEEN 12 INCHES AND 9 FEET SHALL HAVE A MINIMUM OF ONE 18' JOINT OF 150 PSI DUCTILE IRON OR (GREEN) C900 PVC PIPE MEETING ASTM SPECIFICATION D2241 CENTERED ON WATER LINE. WHEN WATER LINE IS
- BELOW SANITARY SEWER PROVIDE MINIMUM 2 FOOT SEPARATION. 9. CONTRACTOR SHALL PROVIDE A MINIMUM HORIZONTAL CLEARANCE OF 9' FEET BETWEEN WATER
- LINES AND SANITARY SEWER MANHOLES AND LINES. 10. SANITARY SEWER MANHOLE RIMS OUTSIDE OF PROPOSED PAVING WILL BE SET 3" - 6" ABOVE THE SURROUNDING LEVEL FINISHED GRADE AFTER PAVING WITH SLOPED BACKFILL ADDED FOR
- 11.IN WET STABLE TRENCH AREAS USE BEDDING PER CITY OF HOUSTON STANDARD DETAILS DRAWING NUMBER 02317-02 (2002).
- 12.DEFLECTION TEST: DEFLECTION TESTS SHALL BE PERFORMED ON ALL FLEXIBLE AND SEMI-RIGID SEWER PIPE. THE TEST SHALL BE CONDUCTED AFTER THE FINAL BACKFILL HAS BEEN IN PLACE AT LEAST 30 DAYS. NO PIPE SHALL EXCEED A DEFLECTION OF 5% IF THE DEFLECTION TEST IS TO BE RUN USING A RIGID MANDREL, IT SHALL HAVE A DIAMETER EQUAL TO 95% OF THE INSIDE DIAMETER OF THE PIPE. THE TEST SHALL BE PERFORMED AS PER 30 TAC 317.2 LATEST AMENDMENT AND WITHOUT MECHANICAL PULLING DEVICES. NO BALL-TYPE MANDREL IS
- 13.INFILTRATION, EXFILTRATION OR LOW-PRESSURE AIR TEST: EITHER OF THE FOLLOWING TESTS SHALL BE PERFORMED AS PER TAC, TITLE 30 317.2 WITHIN THE SPECIFIED TOLERANCES ON ALL
- A. INFILTRATION OR EXFILTRATION TEST: TOTAL LEAKAGE AS DETERMINED BY A HYDROSTATIC HEAD TEST SHALL NOT EXCEED 50 GALLONS PER INCH DIAMETER PER MILE OF PIPE PER 24
- HOURS AT A MINIMUM TEST HEAD OF TWO (2) FEET. B. LOW-PRESSURE AIR TEST: PERFORM TEST ACCORDING TO UNI-B-6-90 OR OTHER APPROPRIATE

  3. NO LANES ON MAJORITY THOROUGHFARES MAY BE BLOCKED FROM 6:00 AM TO 9:00 AM AND 4:00 PROCEDURES. FOR SECTIONS OF PIPE LESS THAN 36" (INCH) AVERAGE INSIDE DIAMETER, THE MINIMUM ALLOWABLE TIME FOR PRESSURE DROP FROM 3.5 P.S.I.G. TO 2.5 P.S.I.G. SHALL BE AS FOLLOWS:
- 6" 340 SECONDS OR 0.855(L) FOR TEST LENGTHS GREATER THAN 398'
- 8" 454 SECONDS OR 1.520(L) FOR TEST LENGTHS GREATER THAN 298'
- 10" 567 SECONDS OR 2.374(L) FOR TEST LENGTHS GREATER THAN 239' 15" 850 SECONDS OR 5 342(L) FOR TEST LENGTHS GREATER THAN 159'
- 18" 1020 SECONDS OR 7.693(L) FOR TEST LENGTHS GREATER THAN 133
- WHERE L = LENGTH OF LINE OF SAME PIPE SIZE IN FEET.

STORM WATER TO DRAIN AWAY FROM MANHOLE RIM.

ALLOWED.

- 14. "SAN. S. E." INDICATES "SANITARY SEWER EASEMENT"
- 15. FOR SANITARY MANHOLE (MH) RIMS SET INSIDE OF OR @ CURB & GUTTER PAVEMENT AND/OR BELOW T.C., MH RIMS WILL BE SET FLUSHED WITH AN ABUTTING PAVED SURFACE. THE (VALCUN, NEENAH OR EQUAL) HEAVY DUTY BOLTED SOLID MH COVER SHALL BE PROPERLY (AND SECURELY)

  9. IF THE CONTRACTOR CHOOSES TO USE A DIFFERENT METHOD OF "TRAFFIC CONTROL PLANS" ATTACHED AND SEALED TO ITS COMPATIBLE GASKETED FRAME BY USING BOTH A NEOPRENE GASKET AND (AT LEAST) 4 COUNTER-SUNK HEX-HEAD COARSE THREADED 1/2"-13 UNC STAINLESS STEEL BOLTS. THE HEAVY DUTY FRAME MH COVER SHALL BE SOLID (NO AIR HOLES). SAID FRAME SHALL BE BOTH EMBEDDED INTO THE MH'S TOP ALSO SECURELY ANCHORED TO THE UNDERLYING MH STRUCTURE WITH EITHER SECURELY ATTACHED EMBEDDED ANCHOR BOLTS OR THE CONCRETE MH'S EXPOSED REBARS WELDED TO THE FRAME OR OTHER EQUALLY SECURED
- METHODS TO PREVENT MH COVER/FRAME BLOW-OFFS/EJECTIONS.

### STORM SEWER CONSTRUCTION NOTES

PRIVATE UTILITY AND OTHER APPURTENANCES.

- 1. STORM SEWER SHALL BE REINFORCED CONCRETE PIPE (C-76, CLASS III), AND SHALL BE INSTALLED, BEDDED, AND BACK FILLED IN ACCORDANCE WITH THE CITY OF HOUSTON DRAWING NOS. 2317-02, 02317-3, 02317-05, 02317-06, AND 02317-07 (OCT. 2002) AS APPLICABLE UNLESS OTHERWISE SHOWN ON THE DRAWINGS
- 2. ALL STORM SEWER CONSTRUCTED IN SIDELOT EASEMENT SHALL BE R.C.P (C-76, CLASSIII) AND SHALL BE EMBEDDED IN ACCORDANCE WITH THE CITY OF HOUSTON DRAWING NOS. 02317-02, 02317-03, 02317-05, 02317-06, AND 02317-07 AS APPLICABLE.
- 3. ALL SEWER UNDER PROPOSED OR FUTURE PAVEMENT AND TO A POINT ONE (1) FOOT BACK OF ALL PROPOSED OR FUTURE CURBS SHALL BE BACKFILLED WITH 1-1/2 SACK CEMENT/C Y STABILIZED SAND TO WITHIN ONE (1) FOOT OF SUBGRADE. THE REMAINING DEPTH OF TRENCH SHALL BE BACKFILLED WITH SUITABLE EARTH MATERIAL
- 4. ALL TRENCH BACKFILL SHALL BE IN 8" LIFTS, WITH TESTS TAKEN AT 100 FOOT INTERVALS IN EACH LIFT, AND MECHANICALLY COMPACTED TO A DENSITY OF NOT LESS THAN 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY THE STANDARD PROCTOR COMPACTION TEST (ASTM D-698/AASHTO T99)
- 5. CIRCULAR AND ELLIPTICAL REINFORCED CONCRETE PIPE SHALL BE INSTALLED USING RUBBER GASKET JOINT CONFORMING TO ASTM C443 AND ASTM C877 RESPECTIVELY.
- 6. ALL STORM SEWER PIPES AND INLET LEADS SHALL BE 24" AND LARGER R.C.P. (C-76, CLASSIII).
- 7. ALL PROPOSED PIPE STUB-OUTS FROM MANHOLES AND INLET LEADS ARE TO BE PLUGGED WITH 8" BRICK WALLS UNLESS OTHERWISE NOTED.
- 8. MINIMUM HORIZONTAL CLEARANCE BETWEEN ANY STORM PIPE AND BOX SHALL BE AT LEAST 48-INCHES FROM EXTERIOR OF THE STORM PIPE OR BOX TO THE EXTERIOR OF THE EXISTING OR PROPOSED PUBLIC OR PRIVATE UTILITY AND OTHER APPURTENANCES. MINIMUM VERTICAL CLEARANCE BETWEEN ANY STORM PIPE AND BOX SHALL BE AT LEAST 18-INCHES FROM EXTERIOR OF THE STORM PIPE OR BOX TO THE EXTERIOR OF THE EXISTING OR PROPOSED PUBLIC OR
- 9. ADJUST MANHOLE COVERS TO GRADE CONFORMING TO REQUIREMENTS OF SECTION
- 02086-ADJUSTING MANHOLES, INLETS, AND VALVE BOXES TO GRADE.
- 10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING, MAINTAINING, AND RESTORING ANY BACK SLOPE DRAINAGE SYSTEM DISTURBED AS A RESULT OF THIS WORK.
- 11. ALL DITCHES SHALL BE GRADED TO PROPOSED ELEVATIONS TO INSURE PROPER DRAINAGE. ALL OUTFALLS SHALL BE PROPERLY BACKFILLED AND COMPACTED. ALL DISTURBED AREA SHALL BE REGRADED. SEEDED. AND FERTILIZED.
- 12. ALL DRIVEWAYS WILL BE LOCATED TO AVOID EXISTING CURB INLET STRUCTURES.

#### PAVING CONSTRUCTION NOTES

- 1. CONTRACTOR SHALL OBTAIN ENGINEERS OR OWNERS APPROVAL OF GRADES PRIOR TO PLACEMENT OF ANY PAVEMENT. IF APPROVAL IS NOT OBTAINED CONTRACTOR SHALL BE RESPONSIBLE FOR PAVEMENT PLACED.
- 2. ALL TEMPORARY AND PERMANENT SIGNAGE MUST COMPLY WITH THE TEXAS MANUAL ON
- UNIFORM TRAFFIC CONTROL DEVICES, MOST RECENT EDITION WITH REVISIONS. 3. ALL PAVING WIDTHS, CURB RADII, AND CURB ALIGNMENT SHOWN INDICATE FACE OF CURB. T.C.
- INDICATES TOP OF CURB. T.P. INDICATES TOP OF PAVEMENT ELEVATIONS. 4. THE CONTRACTOR SHALL PROTECT ALL UTILITIES, SIDEWALKS, PAVEMENT, ETC. AND SHALL
- REPAIR OR REPLACE AT HIS EXPENSE ANY FACILITIES DAMAGED DURING PAVING OR GRADING
- 5. DRIVEWAY CONNECTIONS IN CITY OF HOUSTON STREET RIGHT-OF-WAY SHALL COMPLY WITH CITY OF HOUSTON DRAWING NO. 17201-1 AND DRIVEWAY DETAILS ON THESE PLANS. 6. AREAS TO BE FILLED SHALL BE SCARIFIED AND COMPACTED TO AT LEAST 95% OF MAXIMUM
- STANDARD PROCTOR DENSITY (+3% OF OPTIMUM MOISTURE) PER ASTM D-698 TO A DEPTH OF 6" PRIOR TO FILL PLACEMENT. FILL MATERIAL SHALL BE PLACED IN MAXIMUM 8" THICK LIFTS (MEASURED LOOSE) AND COMPACTED TO AT LEAST 95% OF MAXIMUM STANDARD PROCTOR DENSITY (+ 3% OF OPTIMUM MOISTURE) PER ASTM D-698 FILL SHALL BE CLEAN FARTH AND BE FREE FROM TRASH, VEGETATION AND LARGE STONES. TEST REPORTS SHALL BE SUBMITTED PRIOR TO PLACEMENT OF PAVEMENT.
- REQUIREMENTS SHALL BE DONE BY A COMMERCIAL TESTING LABORATORY ENGAGED BY THE
- CONTRACTOR. 8. WHERE PROPOSED PAVEMENT IS TO MEET EXISTING PAVEMENT. THE EXISTING REBAR OR DOWELS SHALL BE CLEANED AND TIED INTO THE PROPOSED PAVEMENT. USING A MINIMUM OF 30 BAR DIAMETERS LAPS. WHERE PROPOSED CONCRETE ENDS AT A CONSTRUCTION JOINT OR EXPANSION JOINT, THE REBAR SHALL BE EXTENDED A MINIMUM LENGTH OF 30 BAR DIAMETERS, HIS FAILURE TO EXACTLY LOCATE AND PRESERVE THESE UNDERGROUND UTILITIES. COATED WITH ASPHALT AND WRAPPED WITH BURLAP.
- 9. ALL CONCRETE PAVEMENT SHALL BE OF SPECIFIED THICKNESS SHOWN. ALL CONCRETE SHALL BE TO HAVE UNDERGROUND LINES FIELD LOCATED. A 5 % SACK MIX WITH A MINIMUM 500 P.S.I. FLEXURAL STRENGTH AT 7 DAYS AND A MINIMUM 3500. P.S.I. COMPRESSIVE STRENGTH AT 28 DAYS. REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60, UNLESS OTHERWISE NOTED ON THE PLAN.
- 10. ALL CUL-DE-SAC ISLANDS AND MEDIANS SHALL HAVE STANDARD 6"CONCRETE CURBS.
- 11. PUBLIC SIDEWALKS SHALL BE BUILT IN ACCORDANCE WITH CITY OF HOUSTON DESIGN STANDARDS. ALL INTERSECTIONS SHALL BE CONSTRUCTED WITH WHEELCHAIR RAMPS, IN CONFORMANCE WITH THE GOVERNOR'S OFFICE OF TRAFFIC SAFFTY MEMORANDI IM DATED MAY 6 1976 (HIGHWAY SAFETY ACT, 1973, SEC 288). AMERICANS WITH DISABILITIES ACT (ADA) AND TEXAS ACCESSIBILITY STANDARDS (TAS) SHALL BE COMPLIED WITH IN ALL SIDEWALK CONSTRUCTION. 12. CONCRETE WASH-OUT AREAS ARE TO BE PROVIDED BY THE CONTRACTOR AT A LOCATION ACCEPTABLE TO THE OWNER UNDER NO CIRCUMSTANCES IS THE CONTRACTOR TO PERMIT
- CONCRETE TRUCKS TO WASH AT ANY AREA OTHER THAN THAT DESIGNATED. 13. STREET NAME SIGNS SHALL BE BUILT IN ACCORDANCE WITH CITY OF HOUSTON REQUIREMENTS AND SPECIFICATIONS, AND BEAR STREET NAMES AS PER RECORDED PLAT.
- 14. A DOUBLE-REFLECTORIZED BLUE TRAFFIC MARKER SHALL BE PLACED ON A ONE FOOT OFFSET OF THE PAVEMENT CENTERLINE AT ALL FIRE HYDRANT LOCATIONS BY THE CONTRACTOR. HYDRANTS LOCATED AT INTERSECTIONS SHALL HAVE A MARKER PLACED ON EACH STREET THERE WILL BE NO SEPARATE PAYMENT FOR THESE MARKERS.
- 15. DRIVEWAYS WITH TXDOT R.O.W. SHALL BE CONSTRUCTED IN ACCORDANCE WITH TXDOT STANDARDS.
- 16. CURBS ARE 6 INCHES IN HEIGHT.

#### TRAFFIC CONSTRUCTION NOTES

- CONTRACTOR SHALL PROVIDE AND INSTALL TRAFFIC CONTROL DEVICES IN CONFORMANCE WITH PART VI OF THE TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TEXAS MUTCD, MOST RECENT EDITION WITH REVISIONS) DURING CONSTRUCTION.
- 2. NO TRAFFIC LANES SHALL BE CLOSED DURING THE HOURS OF 5:30 AM TO 7:00 PM MONDAY THRU FRIDAY IN DOWNTOWN/MIDTOWN AREA
- PM TO 7:00 PM UNLESS OUTLINED IN THE MOBILITY PERMIT.
- 5. CONTRACTOR SHALL MAINTAIN ONE LANE OF TRAFFIC IN EACH DIRECTION DURING WORKING

4. NO TRAFFIC LANES SHALL BE CLOSED IN RESIDENTIAL AREAS FROM 7:00 PM TO 7:00 AM.

- 6. CONTRACTOR SHALL COVER OPEN EXCAVATIONS WITH STEEL PLATES ANCHORED PROPERLY DURING NON-WORKING HOURS. AND OPEN THE LANES FOR NORMAL TRAFFIC FLOW.
- 7. OFF DUTY UNIFORMED POLICE OFFICER(S)/FLAGGER(S) IS/ARE REQUIRED TO DIRECT TRAFFIC WHEN LANES ARE BLOCKED. 8 IN THE EVENT WHEN NO "TRAFFIC CONTROL PLANS" EXIST AS A PART OF CONTRACT DRAWINGS
- CONTRACTOR MAY PREPARE PLANS\* AND SUBMIT TO THE PLAN REVIEW SECTION FOR APPROVAL TEN WORKING DAYS PRIOR TO IMPLEMENTATION THESE PLANS SHALL BE DRAWN TO SCALE ON REPRODUCIBLE MYLARS AND SEALED BY A LICENSED ENGINEER IN THE STATE OF TEXAS. PLANS WILL BECOME A PART OF THE CONTRACT
- DURING THE CONSTRUCTION THAN WHAT IS OUTLINES IN THE CONTRACT DRAWINGS, HE/SHE SHALL BE RESPONSIBLE FOR PREPARING AND SUBMITTING AN ALTERNATE SET OF PLANS\* TO THE

PLAN REVIEW SECTION FOR APPROVAL TEN WORKING DAYS PRIOR TO IMPLEMENTATION.

10. APPROVED COPIES OF TRAFFIC CONTROL PLANS AND MOBILITY PERMITS SHALL BE MADE AVAILABLE FOR INSPECTION AT THE JOB SITE AT ALL TIMES. CONTRACTORS MUST SECURE MOBILITY PERMITS FROM THE CITY'S TRAFFIC MANAGEMENT AND MAINTENANCE BRANCH BEFORE CLOSING A LANE/SIDEWALK. THE REQUEST MUST BE MADE AT LEAST TEN BUSINESS DAYS IN ADVANCE OF THE CLOSURE NOTE THAT WORKING HOURS MAY BE RESTRICTED OR THE REQUEST MAY BE DENIED. CALL 832-395-3020 FOR AN APPLICATION OR LOG ONTO WWW.GIMS.HOUSTONTX.GOV.

# **SWPPP CONSTRUCTION NOTES**

RECEIVING STORM.

- 1 CONTRACTOR SHALL IMPLEMENT INLET PROTECTION DEVICES AND REINFORCED FILTER FABRIC BARRIER ALONG ROAD AND SIDE DITCHES AT LOCATIONS SHOWN ON THE TYPICAL STORM WATER POLLUTION PREVENTION (SWPPP) PLANS TO KEEP SILT AND/OR EXCAVATED MATERIALS FROM ENTERING INTO THE STORM WATER INLETS AND DITCHES EVENTUALLY POLLUTING THE
- 2. DURING THE EXCAVATION PHASE OF THE PROJECT, CONTRACTOR SHALL SCHEDULE THE WORK IN SHORT SEGMENTS SO THAT EXCAVATED MATERIAL CAN BE QUICKLY HAULED AWAY FROM THE SITE AND TO PREVENT IT FROM STAYING UNCOLLECTED ON THE EXISTING PAVEMENT. ANY LOOSE EXCAVATED MATERIAL WHICH FALL ON PAVEMENTS OR DRIVEWAYS SHALL BE SWEPT BACK INTO THE EXCAVATED AREA.
- 3. CONTRACTOR SHALL CLEAN UP THE EXISTING STREET INTERSECTIONS AND DRIVEWAYS DAILY, AS NECESSARY, TO REMOVE ANY EXCESS MUD, SILT OR ROCK TRACKED FROM THE EXCAVATED
- 4. CONTRACTOR SHALL FOLLOW GOOD HOUSEKEEPING PRACTICES DURING THE CONSTRUCTION OF THE PROJECT. ALWAYS CLEANING UP DIRT AND LOOSE MATERIAL AS CONSTRUCTION PROGRESSES
- 5. CONTRACTOR TO INSPECT AND MAINTAIN THE AREAS LISTED BELOW AT LEAST ONCE EVERY FOURTEEN (14) CALENDAR DAYS AND WITHIN 24 HOURS OF THE END OF A STORM EVENT OF 0.5 INCHES OR GREATER.
- A) DISTURBED AREAS OF THE CONSTRUCTION SITE THAT HAVE NOT BEEN FINALLY STABILIZED. B) AREAS USED FOR STORAGE OF MATERIALS THAT ARE EXPOSED TO PRECIPITATION. C) STRUCTURAL CONTROL MEASURES.
- D) LOCATIONS WHERE VEHICLES ENTER OR EXIT THE SITE.
- 6. CONTRACTOR TO BE RESPONSIBLE TO MAINTAIN EXISTING DITCHES AND/OR CULVERTS FOR UNOBSTRUCTED DRAINAGE AT ALL TIMES. WHERE SODDING IS DISTURBED BY EXCAVATION ON BACKFILLING OPERATIONS, SUCH AREAS SHALL BE REPLACED BY SEEDING OR SODDING. SLOPES 4:1 OR STEEPER SHALL BE REPLACED BY BLOCK SODDING.

#### **PRIVATE UTILITY NOTES**

#### WARNING: OVERHEAD ELECTRICAL FACILITIES

OVERHEAD LINES MAY EXIST ON THE PROPERTY. THE LOCATION OF OVERHEAD LINES HAS NOT BEEN SHOWN ON THESE DRAWINGS AS THE LINES ARE CLEARLY VISIBLE, BUT YOU SHOULD LOCATE THEM PRIOR TO BEGINNING ANY CONSTRUCTION. TEXAS LAW. SECTION 752. HEALTH & SAFETY CODE, FORBIDS ACTIVITIES THAT OCCUR IN CLOSE PROXIMITY TO HIGH VOLTAGE LINES,

- ANY ACTIVITY WHERE PERSON OR THINGS MAY COME WITHIN SIX(6) FEET OF LIVE OVERHEAD HIGH VOLTAGE LINES: AND
- OPERATING A CRANE DERRICK POWER SHOVEL DRILLING RIG. PILE DRIVER HOISTING EQUIPMENT, OR SIMILAR APPARATUS WITHIN 10 FEET OF LIVE OVERHEAD HIGH VOLTAGE LINES.

PARTIES RESPONSIBLE FOR THE WORK, INCLUDING CONTRACTORS, ARE LEGALLY RESPONSIBLE FOR THE SAFETY OF CONSTRUCTION WORKERS UNDER THIS LAW. THIS LAW CARRIES BOTH CRIMINAL AND CIVIL LIABILITY. TO ARRANGE FOR LINES TO BE TURNED OFF OR MOVED, CALL CENTERPOINT ENERGY AT 713-207-2222.

## **ACTIVITIES ON OR ACROSS CENTERPOINT ENERGY FEE OR EASEMENT**

NO APPROVAL TO USE CROSS OR OCCUPY CENTERPOINT FEE OR FASEMENT PROPERTY IS GIVEN. IF YOU NEED TO USE CENTERPOINT PROPERTY. PLEASE CONTACT OUR SURVEYING & RIGHT OF WAY DIVISION AT (713) 207-6348 OR (713) 207-5769.

#### **CAUTION: AT&T TEXAS/SWBT FACILITIES**

1. THE LOCATIONS OF AT&T TEXAS/SWBT FACILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION BEFORE COMMENCING WORK. HE AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY

2. THE CONTRACTOR SHALL CALL 1-800-344-8377 A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION

3. WHEN EXCAVATING WITHIN EIGHTEEN INCHES (18") OF THE INDICATED LOCATION OFAT&T TEXAS/SWBT FACILITIES, ALL EXCAVATIONS MUST BE ACCOMPLISHED USING NON-MECHANIZED EXCAVATION PROCEDURES. WHEN BORING THE CONTRACTOR SHALL EXPOSE THE AT&T TEXAS/SWBT FACILITIES.

4. WHEN AT&T TEXAS/SWBT FACILITIES ARE EXPOSED, THE CONTRACTOR SHOULD PROVIDE SUPPORT TO PREVENT DAMAGE TO THE CONDUIT DUCTS OR CABLES. WHEN EXCAVATING NEAR TELEPHONE POLES THE CONTRACTOR SHALL BRACE THE POLE FOR SUPPORT. 5. THE PRESENCE OR ARSENCE OF AT&T TEXAS/SWBT LINDERGROUND CONDUIT FACILITIES OR

BURIED CABLE FACILITIES SHOWN ON THESE PLANS DOES NOT MEAN THAT THERE ARE NO DIRECT BURIED CABLES OR OTHER CABLES IN CONDUIT IN THE AREA 6. PLEASE CONTACT THE AT&T TEXAS/SWBT DAMAGE PREVENTION MANAGER MR. ROOSEVELT LEE JR. (713)567-4552 OR E-MAIL HIM AT RL7259@ATT.COM, IF THERE ARE QUESTION ABOUT BORING OR

#### EXCAVATING NEAR OUR AT&T TEXAS/SWBT FACILITIES. CAUTION: UNDERGROUND GAS FACILITIES

LOCATIONS OF CENTERPOINT ENERGY MAIN LINES (TO INCLUDE CENTERPOINT ENERGY, INTRASTATE PIPELINE LLC, WHERE APPLICABLE) ARE SHOWN IN AN APPROXIMATE LOCATION ONLY. SERVICE LINES ARE USUALLY NOT SHOWN. OUR SIGNATURE ON THESE PLANS ONLY INDICATES THAT OUR FACILITIES ARE SHOWN IN APPROXIMATE LOCATION. IT DOES NOT IMPLY THAT A CONFLICT ANALYSIS HAS BEEN MADE. THE CONTRACTOR SHALL CONTACT THE UTILITY COORDINATING COMMITTEE AT 1-800-545-6005 OR 811 A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION TO HAVE MAIN AND SERVICE LINES FIELD LOCATED.

WHEN CENTERPOINT ENERGY PIPE LINE MARKINGS ARE NOT VISIBLE, CALL 713-945-8036 OR 713-945-8037 (7:00 AM TO 4:30 PM) FOR STATUS OF LINE LOCATION REQUEST BEFORE EXCAVATION

WHEN EXCAVATING WITHIN EIGHTEEN INCHES (18") OF THE INDICATED LOCATION OF CENTERPOINT ENERGY FACILITIES, ALL EXCAVATION MUST BE ACCOMPLISHED USING NON-MECHANIZED

WHEN CENTERPOINT ENERGY FACILITIES ARE EXPOSED, SUFFICIENT SUPPORT MUST BE PROVIDED TO THE FACILITIES TO PREVENT EXCESSIVE STRESS ON THE PIPING.

FOR EMERGENCIES REGARDING GAS LINES CALL (713) 659-3552 OR (713) 207-4200 THE CONTRACTOR IS FULLY RESPONSIBLE FOR ANY DAMAGES CAUSED BY HIS FAILURE TO EXACTLY LOCATE AND PRESERVE THESE UNDERGROUND FACILITIES.

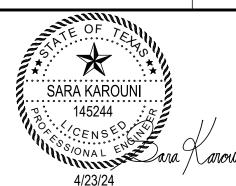
### **ABBREVIATIONS**

_	ABBITE VITTO IN				
Γ	B-B	= BACK TO BACK	NG	= NATURAL GROUND	
	BL	= BUILDING LINE	PROP	= PROPOSED	
	BOC	= BACK OF CURB	ROW	= RIGHT-OF-WAY	
	BBR	= BOTTOM OF RAM	RT	= RIGHT	
	PBW	= BASE OF WALL	R	= RADIUS	
	ESM'T	= EASEMENT	SS	= SANITARY SEWER	
	EX	= EXISTING	SSE	= SANITARY SEWER EASEMENT	
	FL	= FLOWLINE	STM S	WR = STORM SEWER	
	G	= GUTTER	TC	= TOP OF CURB	
	HG	= HYDRAULIC GRADIENT	TG	= TOP OF GRATE	
	HP	= HIGH POINT	TP	= TOP OF PAVEMENT	
	JB	= JUNCTION BOX	TR	= TOP OF RAMP	
	LF	= LINEAR FEET	TW	= TOP OF WALK OR WALL	
	LT	= LEFT	UE	= UTILITY EASEMENT	
1	ME	= MATCH EXISTING	WL	= WATERLINE	
	MH	= MANHOLE	WS&D	= WATER, SANITARY & DRAINAGE	

- 1. WATER LINES, WASTEWATER COLLECTION SYSTEMS, AND STORM DRAINAGE SYSTEMS SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE CITY OF HOUSTON'S DEPARTMENT OF PUBLIC WORKS AND ENGINEERING "DESIGN MANUAL STANDARD CONSTRUCTION SPECIFICATIONS, AND DETAILS FOR WASTEWATER COLLECTION SYSTEMS, WATER LINES, STORM
- DRAINAGE, AND STREET PAVING". 2. ALL STORM SEWER WILL BE REINFORCED CONCRETE (C76 CLASS III) AND SHALL BE INSTALLED, BEDDED AND BACKFILLED IN ACCORDANCE WITH THE CITY OF HOUSTON'S DRAWINGS 02317-02, 02317-03, 02317-05, 02317-06, AND 02317-07 AS APPLICABLE.
- ALL STORM SEWERS CONSTRUCTED IN SIDE LOT EASEMENTS SHALL BE R.C.P., MINIMUM TWENTY (20) FOOT WIDE EASEMENTS SHALL BE PROVIDED. AN ALTERNATIVE TO CEMENT STABILIZED SAND MAY BE USED AS BACKFILL FOR PIPES FIFTY-FOUR (54) INCH AND LARGER, FROM 1-FOOT ABOVE THE TOP OF THE PIPE TO THE BOTTOM OF THE SUBGRADE. CONTRACTOR MAY BACKFILL WITH SUITABLE MATERIAL, PROVIDED THE BACKFILL MATERIAL IS PLACED IN FIGHT (8) INCH LIFTS AND MECHANICALLY COMPACTED TO NINETY-FIVE (95)% STANDARD PROCTOR DENSITY. TESTS SHALL BE TAKEN AT ONE HUNDRED (100) FOOT INTERVALS ON EACH LIFT. BEDDING AND BACKFILL TO ONE (1) FOOT ABOVE THE TOP OF THE PIPE
- SHALL BE CEMENT-STABILIZED SAND. ALL PROPOSED PIPE STUB-OUTS FROM MANHOLES OR INLETS ARE TO BE PLUGGED WITH EIGHT
- (8) INCH BRICK WALLS UNLESS OTHERWISE NOTED. THE CONTRACTOR(S) SHALL NOTIFY HARRIS COUNTY ENGINEERING DEPARTMENT - PERMIT OFFICE TWENTY-FOUR HOURS IN ADVANCE OF COMMENCING UTILITY AND/OR PAVING CONSTRUCTION AT (713) 274 - 3931 AND WRITTEN NOTIFICATION FOURTY-EIGHT (48) HOURS IN ADVANCE OF COMMENCING CONSTRUCTION AT 10555 NORTHWEST FREEWAY, SUITE 144
- HOUSTON, TX 77092. 7. PAVING SHALL BE IN ACCORDANCE WITH THE "REGULATIONS OF HARRIS COUNTY. TEXAS FOR THE APPROVAL AND ACCEPTANCE OF INFRASTRUCTURE" AND/OR AMENDMENTS OF THE SAME. 8. GUIDELINES SET FORTH IN THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" SHALL BE
- OBSERVED 9. OWNER OR OWNER'S AGENT TO OBTAIN ALL PERMITS REQUIRED BY THE "REGULATIONS OF HARRIS COUNTY, TEXAS FOR FLOOD PLAIN MANAGEMENT" PRIOR TO STARTING CONSTRUCTION. 10. OWNER OR OWNER'S AGENT TO OBTAIN ALL NOTIFICATIONS REQUIRED BY HARRIS COUNTY, TEXAS PRIOR TO STARTING CONSTRUCTION OF UTILITIES AND/OR CULVERTS WITHIN HARRIS

COUNTY AND HARRIS COUNTY FLOOD CONTROL DISTRICT RIGHTS-OF-WAY

DESCRIPTION ISSUE FOR CONSTRUCTION 04-23-2024



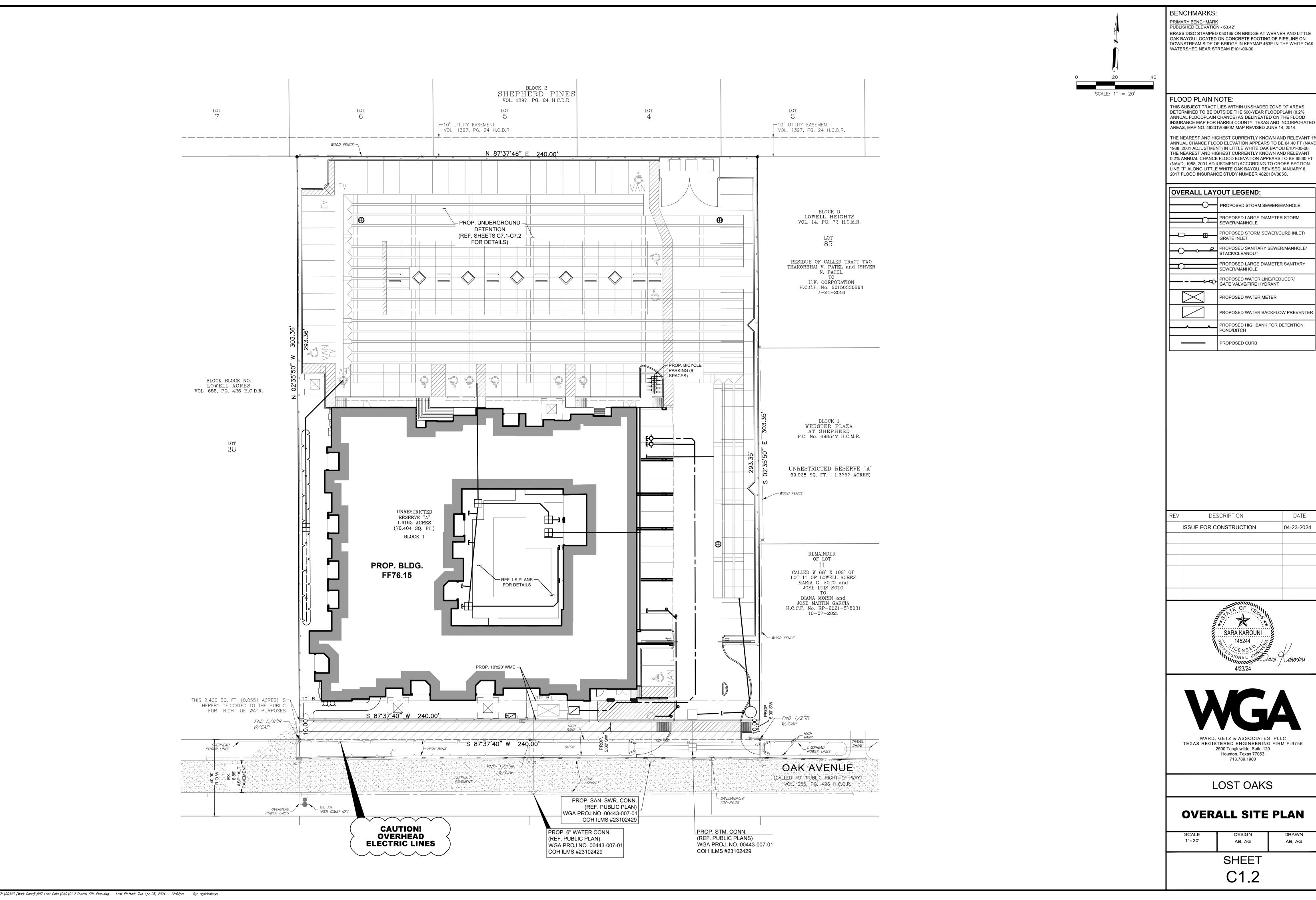


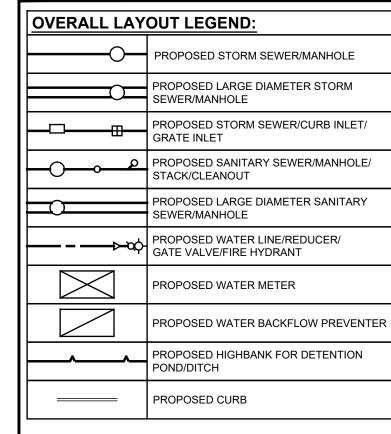
LOST OAKS

# **GENERAL NOTES**

	SHEET	
N/A	AB, AG	AB, AG
SCALE	DESIGN	DRAWN

Drawing: Z:\00443 (Mark Dana)\007 Lost Oaks\CAD\C1.1 General Notes.dwg Last Plotted: Tue Apr 23, 2024 — 12:02pm By: ageldenhuys

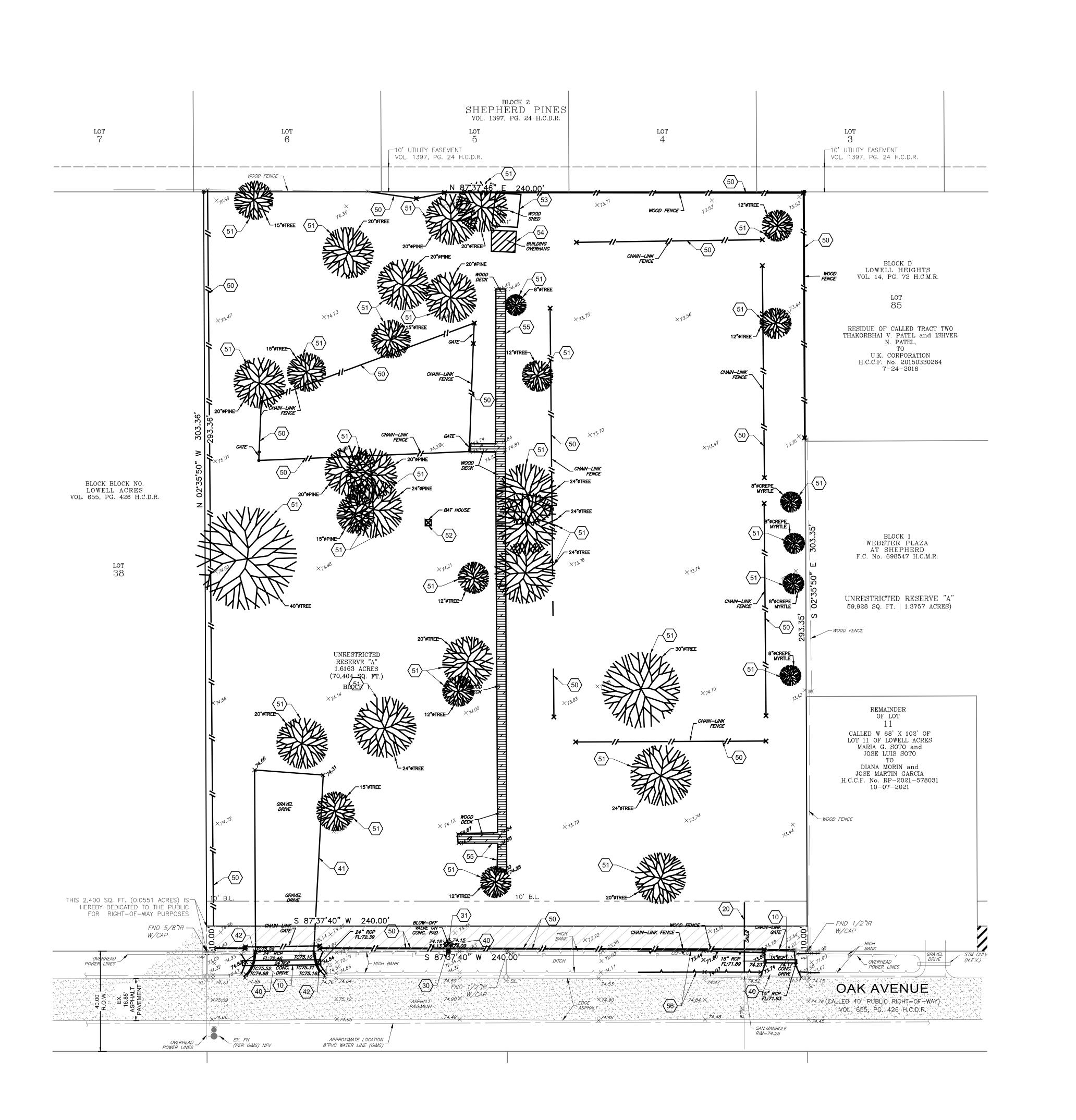




REV	DESCRIPTION	DATE
	ISSUE FOR CONSTRUCTION	04-23-2024
	SEATE OF TENING	

			4
1"=20'	AB, AG	AB, AG	3-007
SOALL	DESIGN	DHAVVIN	4

Drawing: Z: \00443 (Mark Dana)\007 Lost Oaks\CAD\C1.2 Overall Site Plan.dwg Last Plotted: Tue Apr 23, 2024 - 12:02pm By: ageldenhuys



Drawing: Z:\00443 (Mark Dana)\007 Lost Oaks\CAD\C1.3 Demo Plan.dwg Last Plotted: Tue Apr 23, 2024 — 12:02pm By: ageldenhuys

#### BENCHMARKS:

PRIMARY BENCHMARK

PUBLISHED ELEVATION - 63.42'
BRASS DISC STAMPED 050165 ON BRIDGE AT WERNER AND LITTLE
OAK BAYOU LOCATED ON CONCRETE FOOTING OF PIPELINE ON
DOWNSTREAM SIDE OF BRIDGE IN KEYMAP 453E IN THE WHITE OAK
WATERSHED NEAR STREAM E101-00-00

#### FLOOD PLAIN NOTE:

THIS SUBJECT TRACT LIES WITHIN UNSHADED ZONE "X" AREAS DETERMINED TO BE OUTSIDE THE 500-YEAR FLOODPLAIN (0.2% ANNUAL FLOODPLAIN CHANCE) AS DELINEATED ON THE FLOOD INSURANCE MAP FOR HARRIS COUNTY, TEXAS AND INCORPORATED AREAS, MAP NO. 48201V0660M MAP REVISED JUNE 14, 2014.

THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 1% ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 64.40 FT (NAVD 1988, 2001 ADJUSTMENT) IN LITTLE WHITE OAK BAYOU E101-00-00. THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 0.2% ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 65.60 FT (NAVD, 1988, 2001 ADJUSTMENT) ACCORDING TO CROSS SECTION LINE "T" ALONG LITTLE WHITE OAK BAYOU, REVISED JANUARY 6, 2017 FLOOD INSURANCE STUDY NUMBER 48201CV005C.

#### DEMOLITION KEYNOTES

STORM SEWER DEMOLITION:

\$10 STORM SEWER TO BE REMOVED

#### **SANITARY SEWER DEMOLITION:**

SANITARY SEWER LINE TO BE REMOVED

#### WATER DEMOLITION:

(30) WATER METER TO BE REMOVED

BLOW OFF VALVE TO BE REMOVED

#### **PAVEMENT DEMOLITION:**

(40) CONCRETE PAVING TO BE REMOVED

GRAVEL OR CRUSHED CONCRETE TO BE REMOVED

CONCRETE CURB TO BE REMOVED

#### MISCELLANEOUS DEMOLITION:

FENCING TO BE REMOVED

TREE TO BE REMOVED

BAT HOUSE TO BE REMOVED

WOOD SHED TO BE REMOVED

BUILDING OVERHANG TO BE REMOVED

WOOD DECK TO BE REMOVED

DITCH TO BE REMOVED

#### SITE DEMOLITION NOTES:

ALL EXISTING FACILITIES, AS DEPICTED ON THE TOPOGRAPHIC SURVEY, BOTH ABOVE GROUND AND UNDERGROUND, TO BE REMOVED BY CONTRACTOR UNLESS OTHERWISE NOTED. CONTRACTOR TO CONTACT 811 (CITY OF HOUSTON) OR LOCAL EQUIVALENT PRIOR TO DIGGING OR REMOVAL OF ANY WET OR DRY UTILITIES.

CONTRACTOR TO COORDINATE GAS, ELECTRIC, AND COMMUNICATION REMOVAL/RELOCATION WITH THE PROPER PUBLIC & PRIVATE JURISDICTIONAL AGENCIES. WGA DOES NOT GUARANTEE THE LOCATION OR STATUS OF ANY EXISTING UTILITIES SHOWN.

REV	DESCRIPTION	DATE
	ISSUE FOR CONSTRUCTION	04-23-2024
	TE OF TEN	

SARA KAROUNI



WARD, GETZ & ASSOCIATES, PLLC TEXAS REGISTERED ENGINEERING FIRM F-9756 2500 Tanglewilde, Suite 120 Houston, Texas 77063 713.789.1900

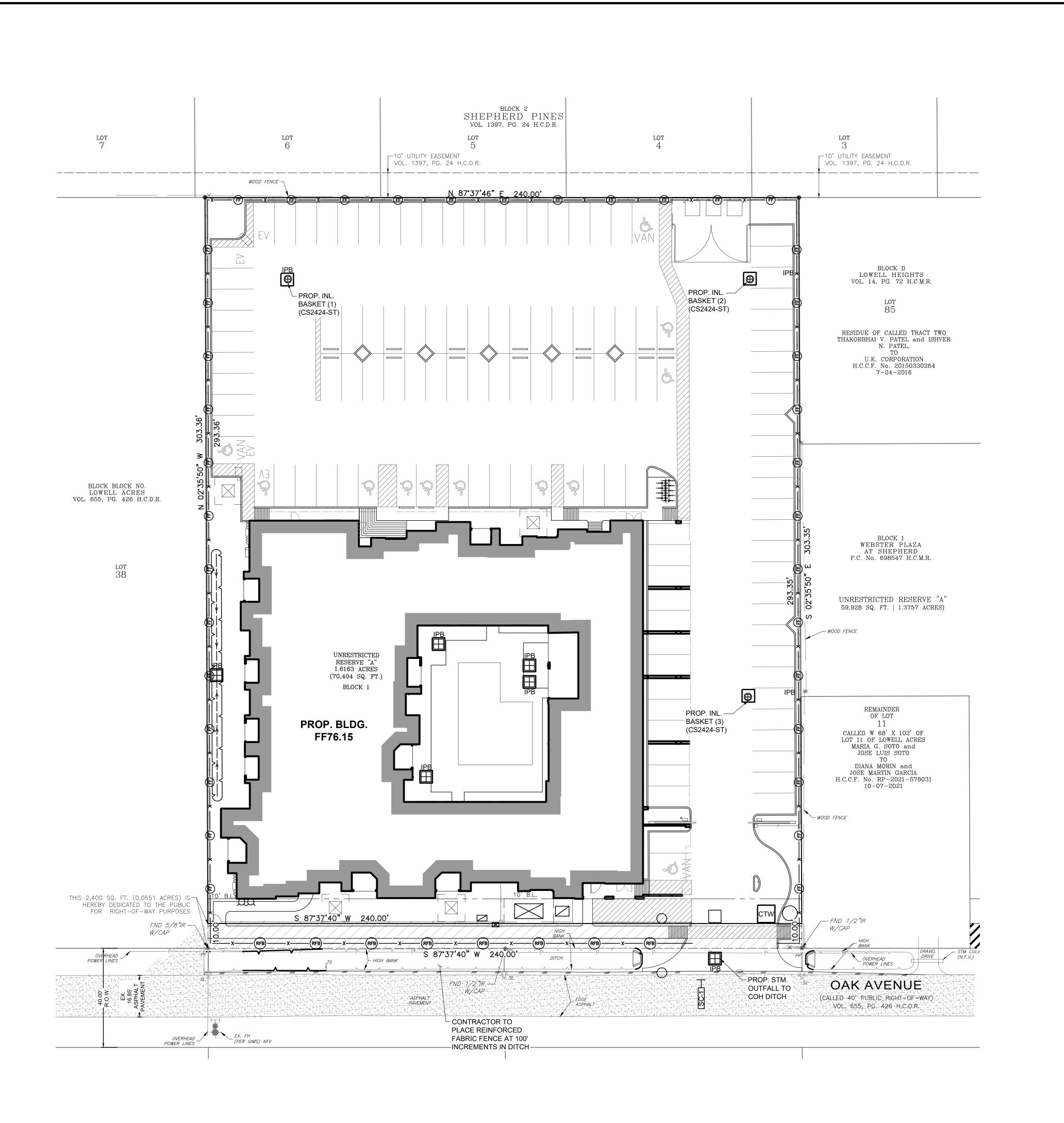
LOST OAKS

## **DEMOLITION PLAN**

SCALE	DESIGN	DRAWN
1"=20'	AB, AG	AB, AG

SHEET

C1.3



**BENCHMARKS**:

SCALE: 1" = 20'

PRIMARY BENCHMARK

PUBLISHED ELEVATION - 63.42' BRASS DISC STAMPED 050165 ON BRIDGE AT WERNER AND LITTLE OAK BAYOU LOCATED ON CONCRETE FOOTING OF PIPELINE ON DOWNSTREAM SIDE OF BRIDGE IN KEYMAP 453E IN THE WHITE OAK WATERSHED NEAR STREAM E101-00-00

#### FLOOD PLAIN NOTE:

THIS SUBJECT TRACT LIES WITHIN UNSHADED ZONE "X" AREAS DETERMINED TO BE OUTSIDE THE 500-YEAR FLOODPLAIN (0.2% ANNUAL FLOODPLAIN CHANCE) AS DELINEATED ON THE FLOOD INSURANCE MAP FOR HARRIS COUNTY, TEXAS AND INCORPORATED AREAS, MAP NO. 48201V0660M MAP REVISED JUNE 14, 2014.

THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 19 ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 64.40 FT (NAVD 1988, 2001 ADJUSTMENT) IN LITTLE WHITE OAK BAYOU E101-00-00. THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 0.2% ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 65.60 FT (NAVD, 1988, 2001 ADJUSTMENT) ACCORDING TO CROSS SECTION LINE "T" ALONG LITTLE WHITE OAK BAYOU, REVISED JANUARY 6, 2017 FLOOD INSURANCE STUDY NUMBER 48201CV005C.

SWPPP LEGEN	ID:
SC-1	STABILIZED CONSTRUCTION EXIT
x	REINFORCED FILTER FABRIC BARRIER
x	FILTER FABRIC FENCE
IPB	INLET PROTECTION BARRIER (STAGE 1 & 2)
CTW	CONCRETE TRUCK WASHOUT AREA

#### EROSION CONTROL SCHEDULE AND SEQUENCING

I. ROUGH GRADING

GRADING CONSTRUCTION ENTRANCE/EXIT, SILT FENCE
PROTECTION, AND STONE OVERFLOW STRUCTURES SHALL BE
INSTALLED PRIOR TO THE INITIATION OF ROUGH GRADING, AS
NEEDED.

### II. UTILITY INSTALLATION

ALL PRIOR EROSION CONTROL MEASURES INSTALLED ABOVE TO BE MAINTAINED AS NECESSARY DURING UTILITY INSTALLATION. INLET PROTECTION SHALL BE INSTALLED AS STORM DRAINAGE SYSTEM IS CONSTRUCTED.

ALL PRIOR EROSION CONTROL MEASURES INSTALLED ABOVE TO BE MAINTAINED AS NECESSARY DURING PAVING AND THROUGHOUT THE REMAINDER OF THE PROJECT.

#### IV. FINAL GRADING/SOIL STABILIZATION/ ALL TEMPORARY EROSION CONTROL

MEASURES TO BE REMOVED AT THE CONCLUSION OF THE PROJECT ONCE FINAL STABILIZATION HAS BEEN ACHIEVED.

#### SWPPP NOTE:

ANY OFF-SITE STAGING AREA UTILIZED BY THE OPERATOR OF THIS SITE MUST BE INCLUDED IN THE SWPPP NARRATIVE PLAN AND TO SWPPP SITE PLAN AS REQUIRED BY THE TPDES TEXAS GENERAL PERMIT TXR150000 (SECTION F). ANY SUCH AREA INCLUDED IN THE SWPPP WILL BE TREATED BY THE OPERATOR AS ANY OTHER PART OF THE CONSTRUCTION ACTIVITY FOR THE PURPOSES OF STORM WATER POLLUTION PREVENTION.

R	EV	DESCRIPTION	DATE
		ISSUE FOR CONSTRUCTION	04-23-2024
	SESSIFIE OF TENT		





WARD, GETZ & ASSOCIATES, PLLC TEXAS REGISTERED ENGINEERING FIRM F-9756 2500 Tanglewilde, Suite 120 Houston, Texas 77063 713.789.1900

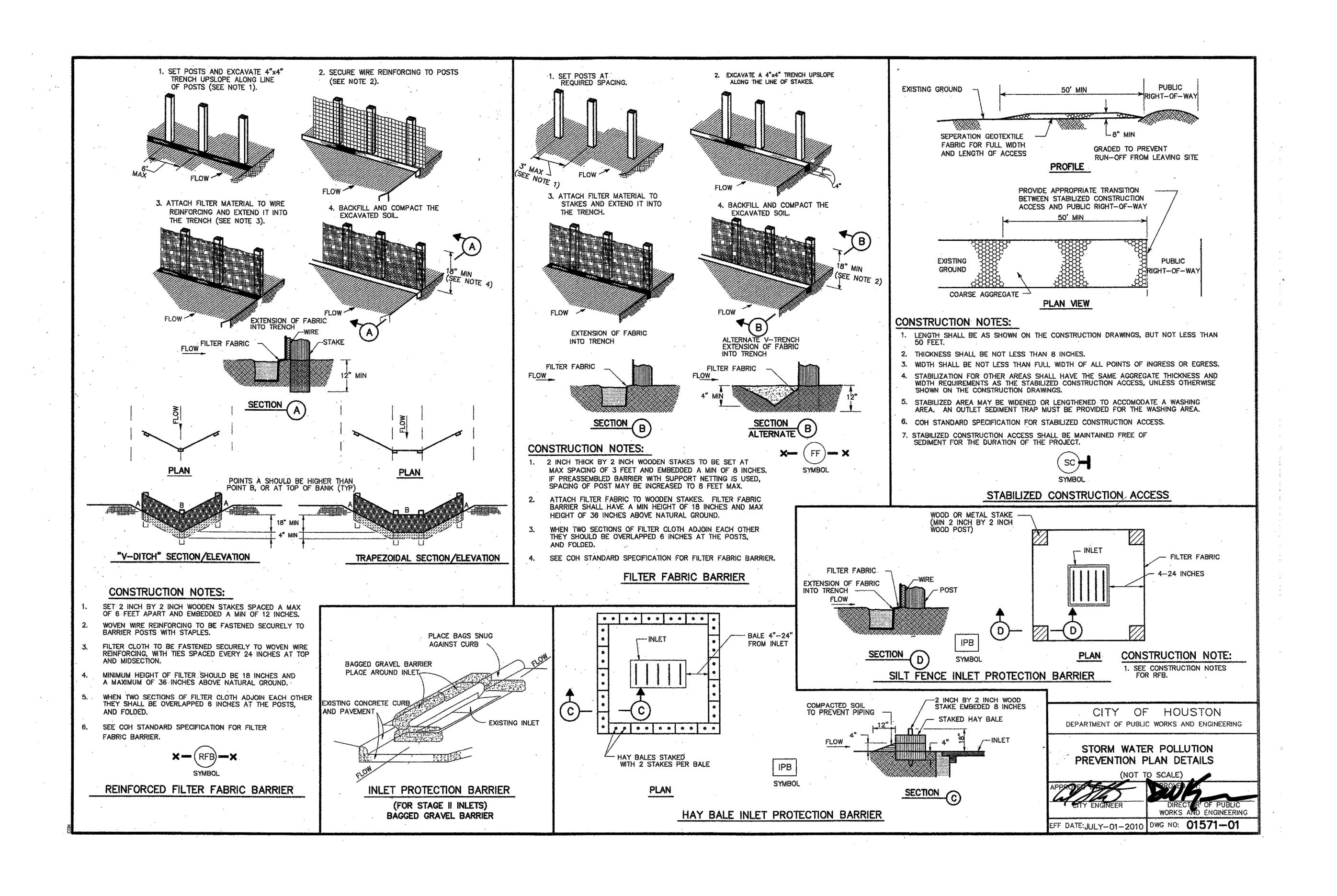
LOST OAKS

#### STORM WATER POLLUTION PREVENTION AND STORM **WATER QUALITY PLAN**

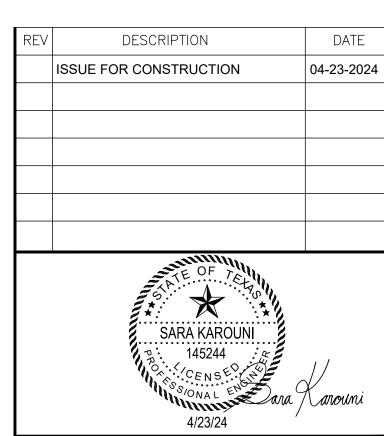
1"=20' AB, AG AB, AG

SHEET

C2.1



Drawing: Z:\00443 (Mark Dana)\007 Lost Oaks\CAD\C2.2 SWPPP and SWQ Details.dwg Last Plotted: Tue Apr 23, 2024 — 12:03pm By: ageldenhuys



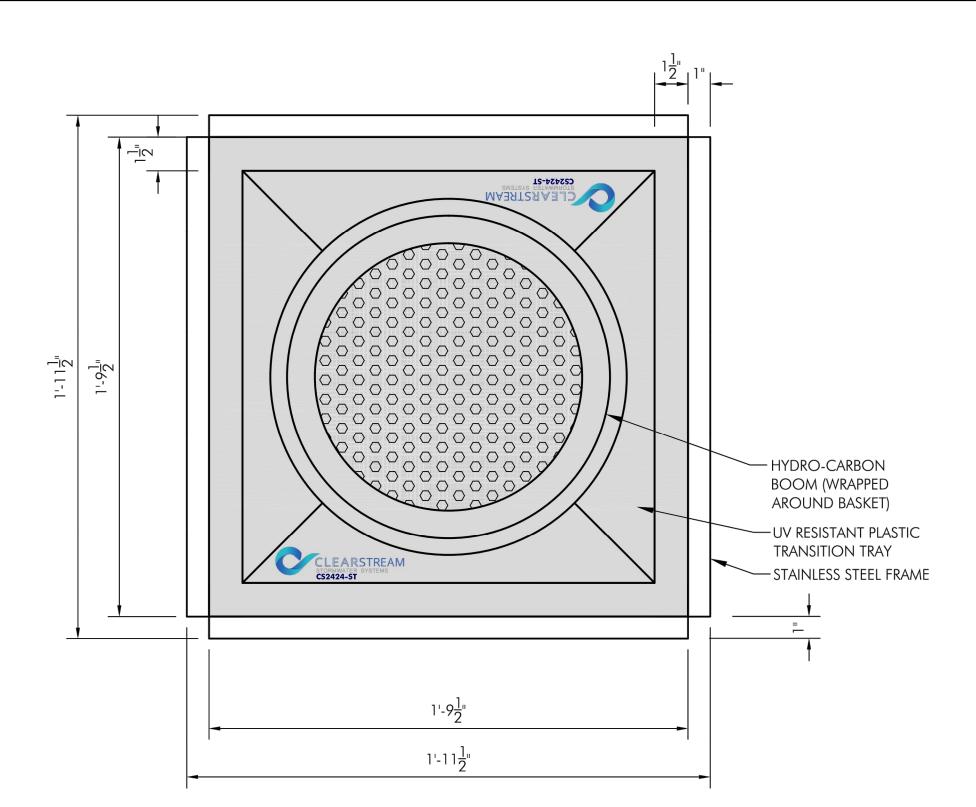


LOST OAKS

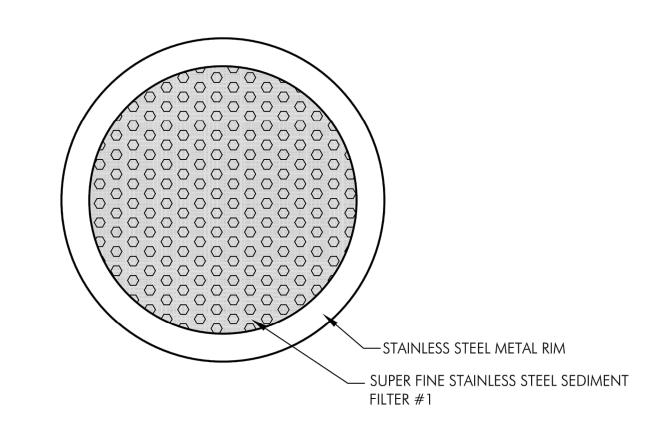
STORM WATER
POLLUTION PREVENTION
PLAN DETAILS

N/A AB, AG AB, AG

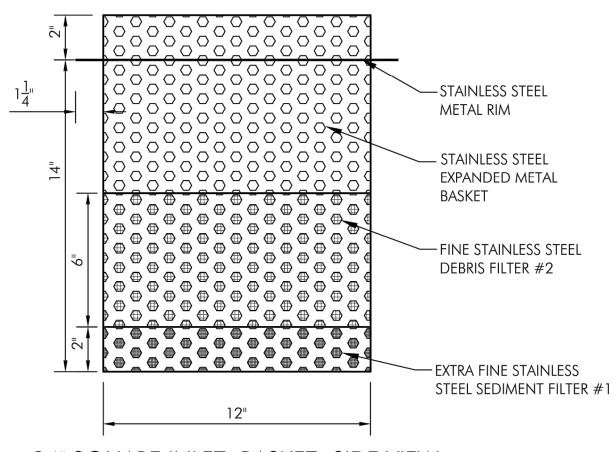
SHEET C2.2



FLOW SPECIFICATIONS				
Q=SO*A*c <sub>d</sub> (2*g*h)^.5	c <sub>d</sub> = Co	efficient o	f Disch	arge= 0.6
	SO	A(ft^2)	h(ft)	Q(ft^3/s
COARSE	73%	0.4583	0.73	1.38
MEDIUM	51%	0.2431	1.23	0.66
FINE	30%	0.1806	1.56	0.33
TREATED FLOW				2.36
OVERFLOW	100%	0.972	0.17	1.91



## 24" SQUARE INLET: BASKET- TOP VIEW



24" SQUARE INLET: BASKET- SIDE VIEW



CLEARSTREAM 24" SQUARE INLET: BASKET

# **Clearstream Inlet Filter Basket**

The Clearstream Inlet Filter Basket is a permanent storm water quality inlet-based filtration device designed to capture silt, sediment, gross pollutants, and hydrocarbons from rainwater runoff. Because these units are installed under the storm grate in parking areas and driveways, they are designed to handle H-20 loading by utilizing 14ga stainless steel frame to support the unit. Framing members are welded for additional support and structural integrity. The transition tray is fabricated with a single ¼" thick vacuum molded UV resistant plastic which is both rigid and light designed to support the filter basket without adding any unnecessary weight. Additionally, the tray includes a recessed area to accept a standard 3" hydrocarbon boom, eliminating the need to use any proprietary filtration devices to satisfy hydrocarbon filtration required by most storm water regulations. The filter basket is constructed of 304 stainless steel which will not rust, corrode or warp under normal use. All screens are made of 304 stainless to create a basket that will last for decades if properly maintained. All Clearstream filters and associated components carry a 5-year warranty against manufacturing defects.

DESCRIPTION ISSUE FOR CONSTRUCTION

ST

CS2424-

**MODEL:** 

ORMWATE

FOR REFERENCE ONLY

04-23-2024



LOST OAKS

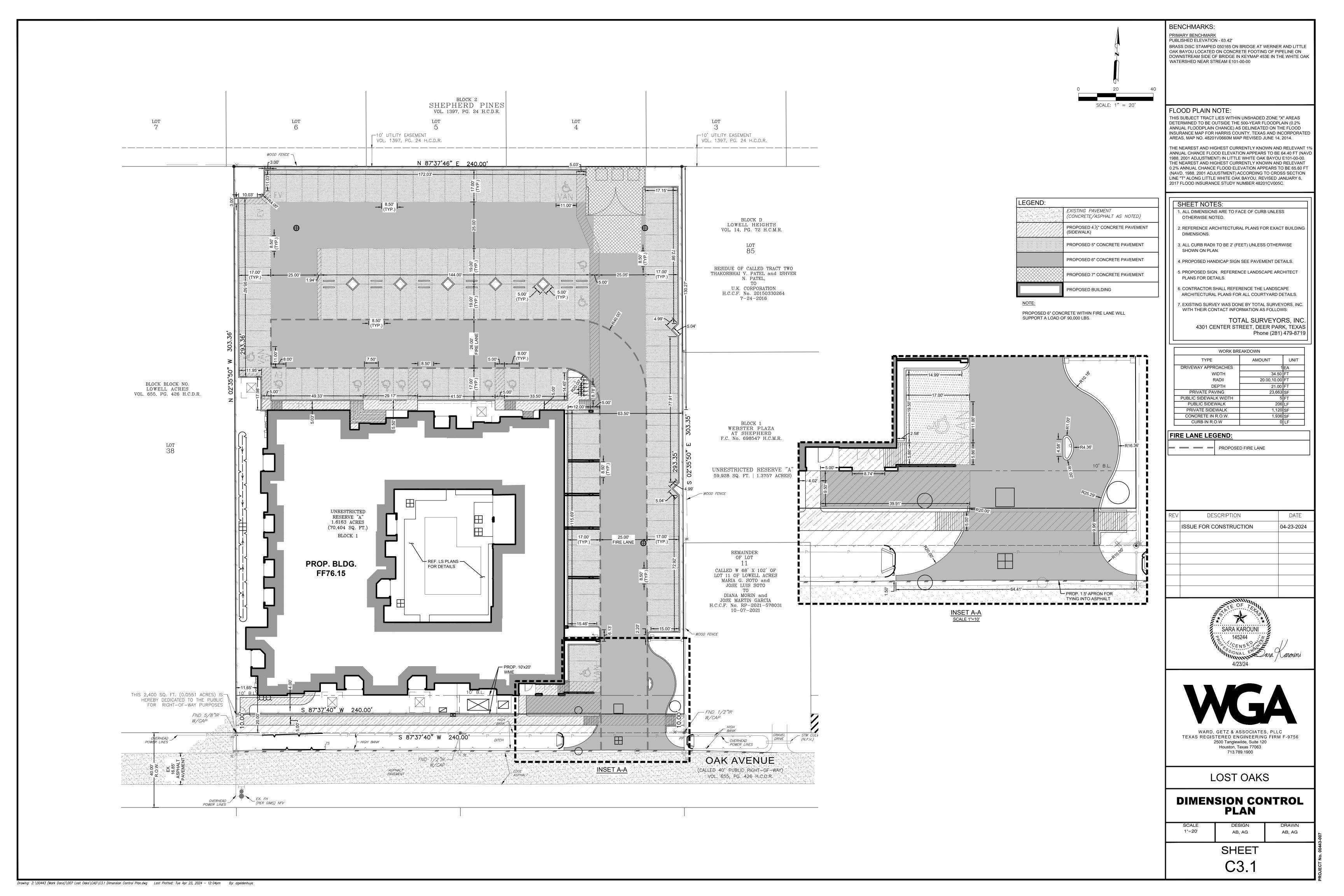
STORM WATER QUALITY FEATURE DETAIL

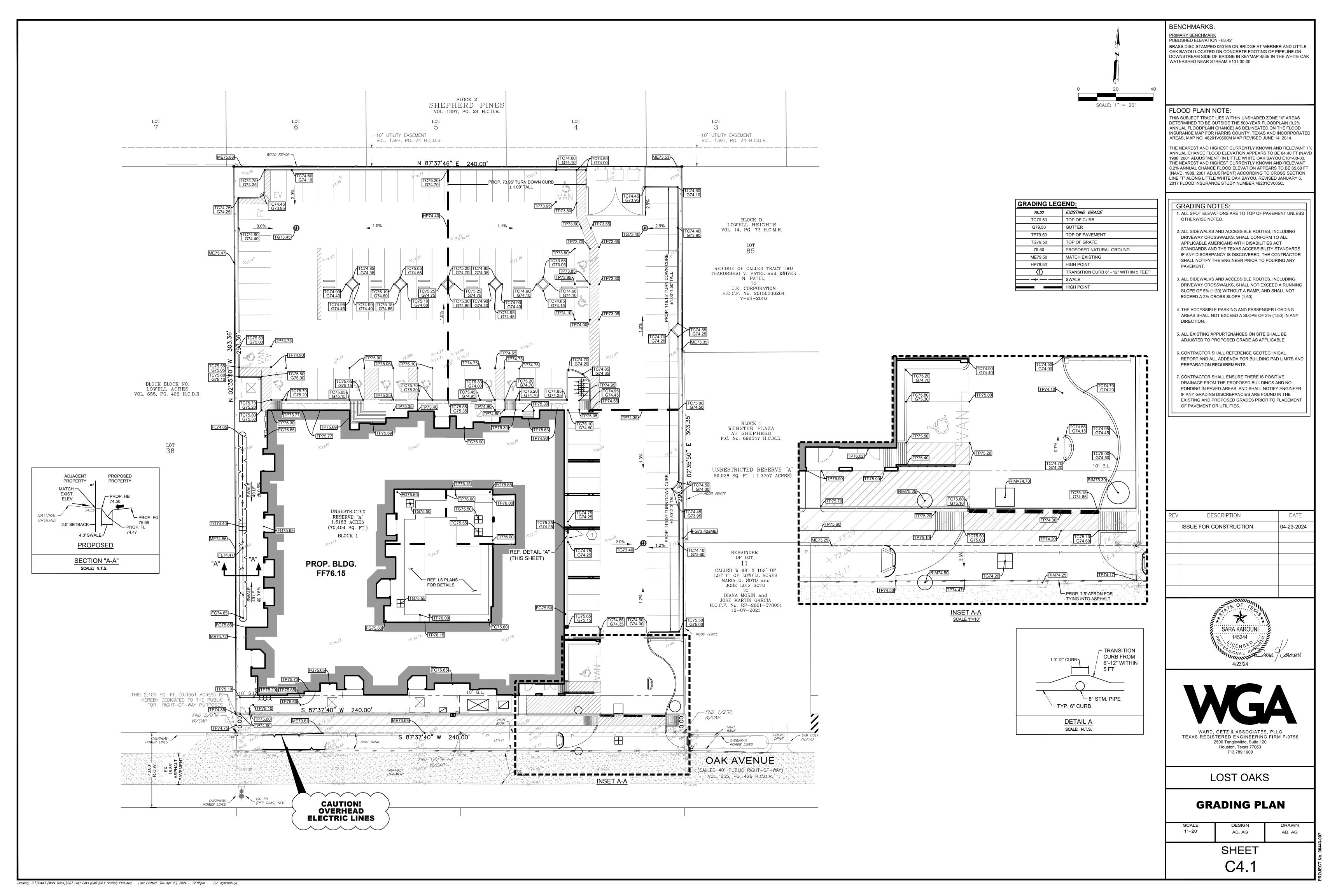
AB, AG AB, AG

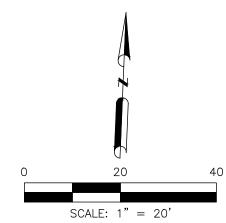
SHEET C2.3

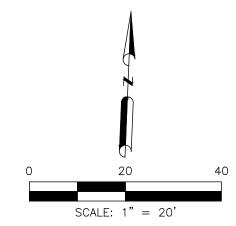
	-	1'-11 <u>1</u> "	EXTREME EVENT OVERFLOW OPENINGS
HYDRO-CARBON — BOOM			STAINLESS STEEL FRAME  UV RESISTANT PLASTIC  TRANSITION TRAY
	114"		STAINLESS STEEL  METAL RIM  STAINLESS STEEL  EXPANDED METAL  BASKET
	= 9		FINE STAINLESS STEEL DEBRIS FILTER #2
CLEARSTREAM STORMWATER SYSTEMS	_ <u> </u> ≅  24" SQUA	RE INLET- SECTION : IN	EXTRA FINE STAINLESS STEEL SEDIMENT FILTER #1

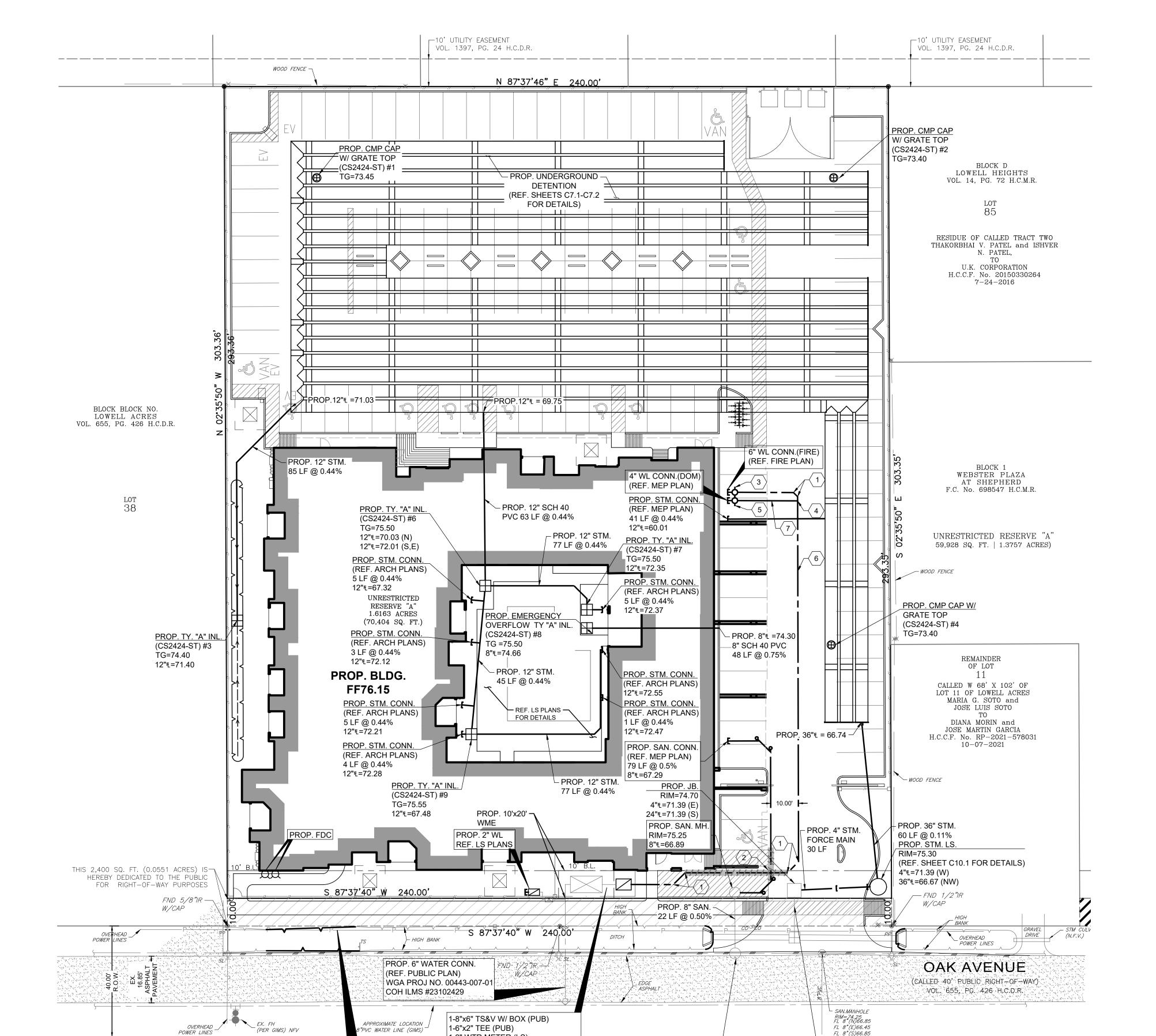
CLEARSTREAM 24" SQUARE INLET-PLAN VIEW: INSTALLED











APPROXIMATE LOCATION

(PER GIMS) NFV

**\\\\** 

**CAUTION!** 

**OVERHEAD** 

**ELECTRIC** 

1-6"x2" TEE (PUB)

1-2" PLUG (PUB)

1-2" PLUG (PVT)

1-6" PLUG (PVT)

1-6" RPZ BFP (PVT)

1-90° 2" BEND (PVT)

1-2" RPZ BFP (PVT)

1-6" 90° BEND (PUB)

1-6" WTR METER (FIRE/DOM) (PUB)

1-2" WTR METER (LS) 1-90° 2" BEND (PUB)

PROP. 24" RCP STM.

(REF. PUBLIC PLANS)

COH ILMS #23102429

17 LF @ 0.18%

WGA PROJ. NO.

00443-007-01

PROP. SAN. MH.

RIM=MATCH PVMT

PROP. 8"f=66.78 (N)

EX. 8"£=66.78 (E,W)

WGA PROJ. NO.

00443-007-01

(REF. PUBLIC PLANS)

COH ILMS #23102429

PIPING MATERIAL SCHEDULE	
SANITARY SEWER - 6" AND SMALLER	SCHEDULE 40 PVC
SANITARY SEWER - 8" AND LARGER	SDR-26 PVC
WATER LINE - SMALLER THAN 4"	SCHEDULE 40 PVC AS PER ASTM D1785
WATER LINE - 4" TO 12"	PVC CLASS 150, DR-18, AWWA C-900
STORM SEWER - SMALLER THAN 12"	SDR-35 PVC PER ASTM D3034
STORM SEWER - 12" OR LARGER (PRIVATE)	DUAL-WALLED HDPE, AASHTO M252 & M294
STORM SEWER - WITHIN PUBLIC EASEMENT OR RIGHT-OF-WAY	RCP, C-76, CLASS III
STORM SEWER - UNDERNEATH BUILDING	SCHEDULE 40 PVC

#### NOTE:

CLEARSTREAM INLET BASKETS (CS2424-ST) TO BE UTILIZED AT ALL TYPE "A" INLETS AS STORM WATER QUALITY FEATURE

WATER CALL	WATER CALLOUTS:	
1	6" 45° BEND	
2	WATER / SANITARY CROSSING (SEE SHEET NOTE 9)	
3	6" GATE VALVE & BOX	
4	6"x4" TEE	
5	4" GATE VALVE & BOX	
6	6" WATER LINE (FIRE)	
7	4" WATER LINE (DOMESTIC)	

#### **BENCHMARKS**:

PRIMARY BENCHMARK

PUBLISHED ELEVATION - 63.42' BRASS DISC STAMPED 050165 ON BRIDGE AT WERNER AND LITTLE OAK BAYOU LOCATED ON CONCRETE FOOTING OF PIPELINE ON DOWNSTREAM SIDE OF BRIDGE IN KEYMAP 453E IN THE WHITE OAK WATERSHED NEAR STREAM E101-00-00

#### FLOOD PLAIN NOTE:

THIS SUBJECT TRACT LIES WITHIN UNSHADED ZONE "X" AREAS DETERMINED TO BE OUTSIDE THE 500-YEAR FLOODPLAIN (0.2% ANNUAL FLOODPLAIN CHANCE) AS DELINEATED ON THE FLOOD INSURANCE MAP FOR HARRIS COUNTY, TEXAS AND INCORPORATED AREAS, MAP NO. 48201V0660M MAP REVISED JUNE 14, 2014.

THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 19 ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 64.40 FT (NAVD 1988, 2001 ADJUSTMENT) IN LITTLE WHITE OAK BAYOU E101-00-00. THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 0.2% ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 65.60 FT (NAVD, 1988, 2001 ADJUSTMENT) ACCORDING TO CROSS SECTION LINE "T" ALONG LITTLE WHITE OAK BAYOU, REVISED JANUARY 6, 2017 FLOOD INSURANCE STUDY NUMBER 48201CV005C.

#### **UTILITY NOTES:**

SEE LANDSCAPE PLANS.

PAVING.

APPLICABLE.

- 1. PRIVATE STORM SEWER TO BE HDPE UNLESS OTHERWISE NOTED. FOR PUBLIC STORM SEWER REFERENCE GENERAL
- 2. UTILITIES SHOWN ARE APPROXIMATE. CONTRACTOR TO FIELD VERIFY EXISTING UTILITY LOCATIONS PRIOR TO CONSTRUCTION AND IMMEDIATELY NOTIFY ENGINEER OF ANY CONFLICT OR DISCREPANCIES.
- 3. CONTRACTOR TO COORDINATE WITH UTILITY COMPANIES FOR SERVICE ORIGINATION AND CONNECTION.
- 4. CONTRACTOR SHALL TAKE CARE NOT TO DAMAGE ANY EXISTING ITEMS ONSITE.
- 5. CONTRACTOR SHALL KEEP THE SITE CLEAN OF DEBRIS AND ANY EROSION CONTROL MEASURES ARE
- ADEQUATELY PLACED. 6. CONTRACTOR TO COORDINATE LOCATIONS OF UNDERGROUND IRRIGATION SLEEVING PRIOR TO PAVING.
- 7. CONTRACTOR TO COORDINATE LOCATIONS OF UNDERGROUND CONDUIT FOR SITE LIGHTING PRIOR TO
- 8. ALL CONSTRUCTION OPERATIONS SHALL BE ACCOMPLISHED IN ACCORDANCE WITH APPLICABLE REGULATIONS OF THE U.S. OCCUPATIONAL HEALTH AND SAFETY ADMINISTRATION. COPIES OF OSHA STANDARDS MAY BE PURCHASED FROM THE U.S. GOVERNMENT PRINTING OFFICE. INFORMATION AND RELATED REFERENCE MATERIALS MAY BE PURCHASED FROM OSHA, 903 SAN JACINTO, RM 319, AUSTIN, TX. 78701. TEL: (512)
- 9. ALL SANITARY SEWERS CROSSING WATER LINES WITH A 6 INCHES TO 9 FEET CLEARANCE SHALL HAVE A MINIMUM OF 18' JOINT OF 150 P.S.I. SDR 26 PVC PIPE MEETING ASTM SPECIFICATION D2241 CENTERED ON WATER LINE. WHEN WATER LINE IS BELOW SANITARY SEWER PROVIDE MINIMUM 2 FOOT SEPARATION.
- 0. REFERENCE MEP PLANS FOR UTILITY CONDUI LOCATIONS.
- 11. CONTRACTOR TO ADJUST ALL EXISTING APPURTENANCES ON SITE TO PROPOSED GRADE, AS

REV	DESCRIPTION	DATE
	ISSUE FOR CONSTRUCTION	04-23-2024
	SARA KAROUNI	



WARD, GETZ & ASSOCIATES, PLLC TEXAS REGISTERED ENGINEERING FIRM F-9756 2500 Tanglewilde, Suite 120 Houston, Texas 77063

713.789.1900

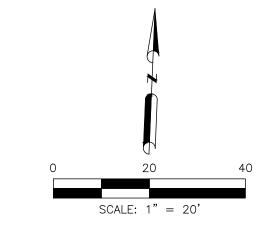
LOST OAKS

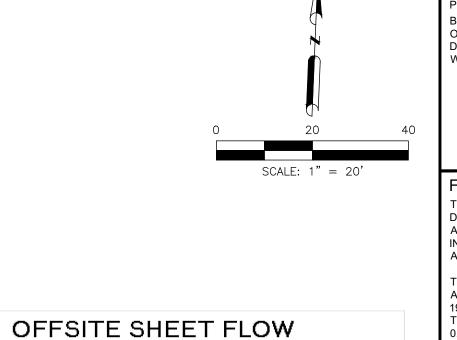
# **UTILITY PLAN**

1"=20' AB, AG AB, AG

> SHEET C5.1

Drawing: Z:\00443 (Mark Dana)\007 Lost Oaks\CAD\C5.1 Utility Plan.dwg Last Plotted: Tue Apr 23, 2024 — 12:06pm By: ageldenhuys





Rainfall Frequency: 100

 $i = \frac{1}{(d+TC)^e}$   $d = \frac{4.44}{6}$ 

 $TC = 10A^{0.1761} + 15$ 

C C\*Cf min. (In / Hr.)

0.18 0.18 **25.22** 8.49

0.80 0.80 **20.67** 9.35 0.30

e = 0.58

Cf = 1

1.73

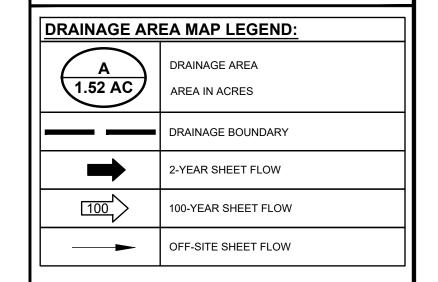
#### **BENCHMARKS**: PRIMARY BENCHMARK PUBLISHED ELEVATION - 63.42'

BRASS DISC STAMPED 050165 ON BRIDGE AT WERNER AND LITTLE OAK BAYOU LOCATED ON CONCRETE FOOTING OF PIPELINE ON DOWNSTREAM SIDE OF BRIDGE IN KEYMAP 453E IN THE WHITE OAK WATERSHED NEAR STREAM E101-00-00

#### FLOOD PLAIN NOTE:

THIS SUBJECT TRACT LIES WITHIN UNSHADED ZONE "X" AREAS DETERMINED TO BE OUTSIDE THE 500-YEAR FLOODPLAIN (0.2% ANNUAL FLOODPLAIN CHANCE) AS DELINEATED ON THE FLOOD INSURANCE MAP FOR HARRIS COUNTY, TEXAS AND INCORPORATED AREAS, MAP NO. 48201V0660M MAP REVISED JUNE 14, 2014.

THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 19 ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 64.40 FT (NAVD 1988, 2001 ADJUSTMENT) IN LITTLE WHITE OAK BAYOU E101-00-00. THE NEAREST AND HIGHEST CURRENTLY KNOWN AND RELEVANT 0.2% ANNUAL CHANCE FLOOD ELEVATION APPEARS TO BE 65.60 FT (NAVD, 1988, 2001 ADJUSTMENT) ACCORDING TO CROSS SECTION LINE "T" ALONG LITTLE WHITE OAK BAYOU, REVISED JANUARY 6, 2017 FLOOD INSURANCE STUDY NUMBER 48201CV005C.



		T
REV	DESCRIPTION	DATE
	ISSUE FOR CONSTRUCTION	04-23-2024
	SARA KAROUNI	



713.789.1900

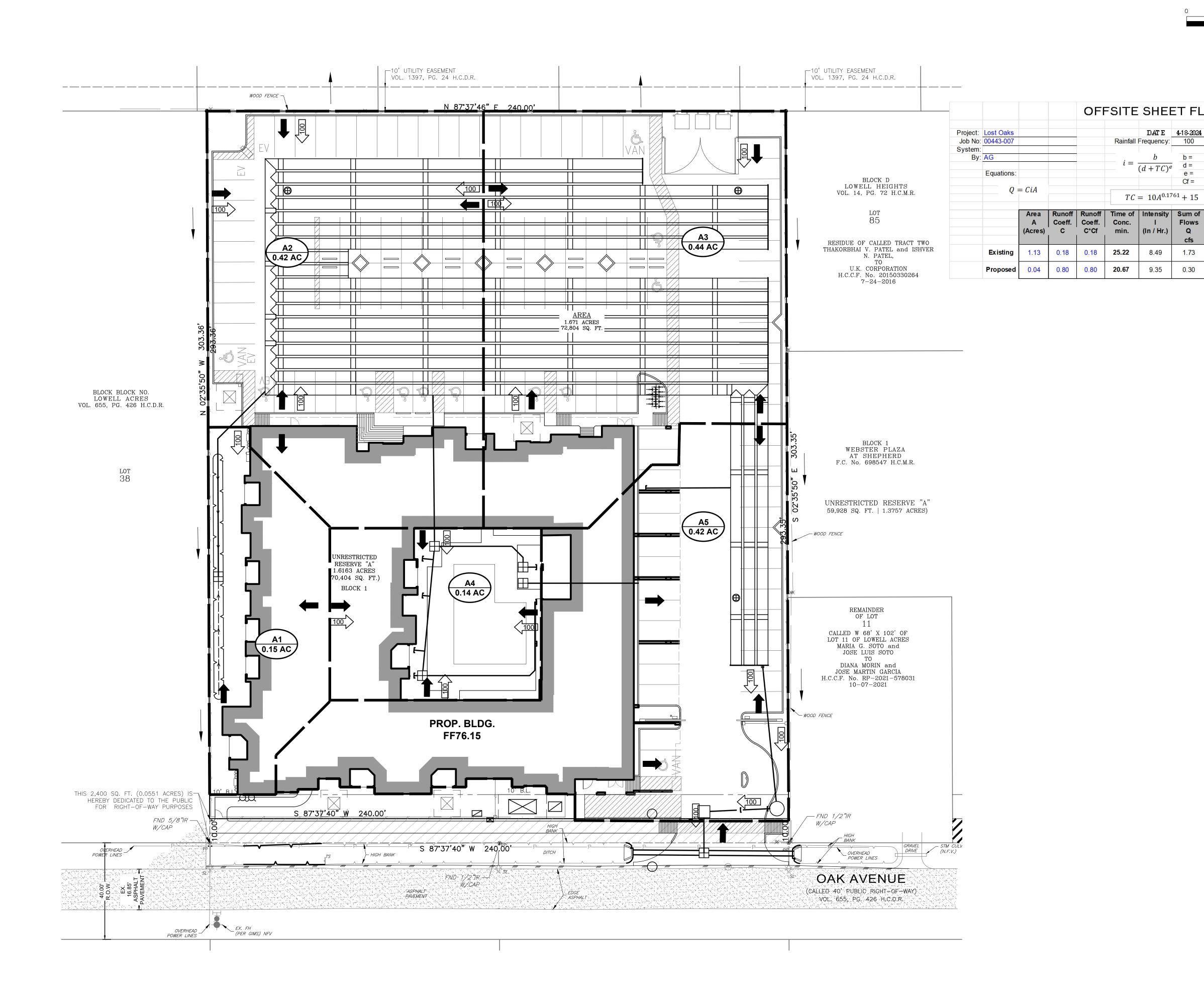
2500 Tanglewilde, Suite 120 Houston, Texas 77063

LOST OAKS

# **DRAINAGE AREA MAP**

ľ		OUEET	
	1"=20'	AB, AG	AB, AG
	SCALE	DESIGN	DRAWN

SHEET C6.1



 $\textit{Drawing: Z: $\setminus 0.0443 (Mark Dana) \setminus 0.07 Lost Oaks \setminus CAD \setminus C6.1 Drainage Area Map.dwg } \quad \textit{Last Plotted: Tue Apr 23, 2024 - 12:06pm} \quad \textit{By: ageldenhuys}$ 

	WG	λí																														
								9	STORM	SEW	ER CA	LCUI	ATIO	NS																		
																	C <sub>f</sub> =	1.00														
		Project:	LostOal	(S							DATE	4-19-2024	1	i -	b		b =	48.35														
		Job No	: 00443-0	07					Rainfa	II Frequency	(years):	2			(d+TC)		d =	9.07														
		System	i: <b>A</b> ,B												,		e =	0.72														
		Ву	/: AG																													
	#N/A																															
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32
													LINE	0.0			DE	SIGN				HYD. GRAD			]						•	
			Sub	Total	Sub	Sub		Time of	Intensity	Sum of	Reach	Dia.	# of	Width					drop at	Actual	Hydraulic	Change	Elev.	Elev.								
ID	From	То	Area	Area	Runoff	Runoff	Sum of	Conc.	I	Flows	Length	in.	Pipes/	of	Slope M	lannings	Capacity	Velocity	down	Velocity	Gradient	in Head	Up	Down	TG/ Gutter	TG/ Gutter						
10	МН	MH	Acres	Acres	Coeff.	Coeff.	Cf*C*A	min.	(In / Hr.)	cfs	Feet	or Rise	Boxes	box	%	n"	cfs	fps	stream	fps	%	Ft.	Stream	Stream	Elevation	Elevation	МН	flowline	МН	flowline		Top of Pipe
					С	C*C <sub>f</sub>				Q		(ft)		(ft)			Q		manhole						Upstream	Downstream	ı UP	upstream	DOWN	downstream	(upstream)	(downstream
			0.45	0.45	0.00	0.00	0.40	00.40	4.00	0.40	0.5	40			0.44	0.010	0.57	0.07	0.00	0.40	0.045	0.040	00.00	00.07	74.40			74.40		74.00	70.40	70.00
A.	1	Pond	0.15	0.15	0.80	0.80	0.12	22.16	4.00	0.48	85	12	1		0.44	0.012	2.57	3.27	0.00	2.49	0.015	0.013	69.69	69.67	74.40	-	1 1	71.40	Pond	71.03	72.40	72.03
" A "		Dond	0.42	0.42	0.00	0.80	0.34	23.58	3.87	1.30	0	12	1		0.44	0.012	2.57	3.27	0.00	3.27	0.113	0.000	69.67	*TOP OF F	73.40		2	66.74	Pond	66.74	67.74	67.74
A		Pond	0.42	0.42	0.80	0.60	0.34	23.30	3.01	1.30	U	12	1		0.44	0.012	2.37	3.21	0.00	3.21	0.113	0.000	09.07	*TOP OF F		-	2	00.74	Pond	00.74	67.74	07.74
"Δ'	3	Pond	0.44	0.44	0.80	0.80	0.35	23.65	3.86	1.36	0	12	1		0.44	0.012	2.57	3.27	0.00	3.30	0.124	0.000	69.67	69.67	74.40	_	3	68.70	Pond	68.70	69.70	69.70
		1 Olid	0.44	0.44	0.00	0.00	0.00	20.00	3.00	1.00	U	12			0.44	0.012	2.01	0.21	0.00	0.00	0.124	0.000	00.07	*TOP OF F			0	00.70	1 Ond	00.70	00.70	00.70
"A'	4	Pond	0.14	0.14	0.80	0.80	0.11	22.07	4.00	0.45	63	12	1		0.44	0.012	2.57	3.27	0.00	2.44	0.014	0.009	69.68	69.67	75.50	-	4	71.97	Pond	71.69	72.97	72.69
																								*TOP OF F	PIPE			AC 101100000				
"A'	5	Pond	0.42	0.42	0.80	0.80	0.34	23.58	3.87	1.30	0	12	1		0.44	0.012	2.57	3.27	0.00	3.27	0.113	0.000	69.67	69.67	73.40	-	5	71.69	Pond	71.69	72.69	72.69
																								*TOP OF F	PIPE							
"B'	Pond	LS	0.00	1.57	0.80	0.80	1.26	25.83	3.69	4.63	60	36	1			0.012	25.10	3.55	0.00	2.70	0.004	0.002	69.67	69.67	-	75.30	Pond	66.74	LS	66.67	69.74	69.67
"B'	LS	MH1	0.00	1.57	0.80	0.80	1.26	26.11	3.67	4.63	32	24	1			0.012	10.43	3.32	0.00	3.20	0.036	0.011	68.67	73.39	75.30	74.20	LS	66.67	MH1	71.39	68.67	73.39
"B'	MH1	OUT	0.00	1.57	0.80	0.80	1.26	26.27	3.65	4.63	24	24	1		0.18	0.012	10.43	3.32	0.00	3.20	0.036	0.009	73.36	73.35	74.20	74.50	MH1	71.39	OUT	71.35	73.39	73.35
	611	D.C.	6.55	0.00	0.00	0.00	0.07	04.51		0.00					0.75	0.0/-		0.55	0.55	0.70	0.550		06.70	00.07	70.10		611	71.35	D.C. I.E.	7.55	75.00	7, 57
"E"	CY	POND	0.09	0.09	0.80	0.80	0.07	21.54	4.06	0.29	48	8	1		0.75	0.012	1.14	3.26	0.00	2.70	0.050	0.024	69.70	69.67	76.10	=	CY	74.66	POND	74.30	75.33	74.97

4	WG	Α																													
								9	TORM	SEW	ER CA	LCUL	ATIC	NS																	
																C <sub>f</sub> =	1.00														
		Project:	Lost Oak:	S							DATE	4-19-2024		i =	b	b =	60.66														
		Job No:		)7					Rainfa	II Frequency	(years):	100		. (d	$+TC)^e$	d =	4.44														
		System:														e =	0.58														
0	.\.	By:	AG	a al\ Mia y a a	££\\ A fin al avec a	\I\latCaaba	Content MC	O)[Comy of Lo	at Oaka Otawa Oa	las visiDECICI	\  (O , , a a #)																				
C.	.vosers ag	eidennuysv	<b>ч</b> рр⊿а≀а\Lα	ocal\iviici osc	TILLYVINGOW S	inelCache	Content.WS	Olicopy of Fo	st Oaks Storm Ca	iics.xisju <del>e</del> SiGi	v(∠-year)																				
	1	2	3	4	5	6	7	8	9	10	11	12	13	14 1	5 16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32
													LINE			D	SIGN				HYD. GRAD										
			Sub	Total	Sub	Sub		Time of	Intensity	Sum of	Reach	Dia.	# of	Width				drop at	Actual	Hydraulic	Change	Elev.	Elev.								
ID _	From	То	Area	Area	Runoff			Conc.	I	Flows	Length	in.	Pipes/	of SIC	pe Manni	ngs Capacit	Velocity	down	Velocity	Gradient	in Head	Up	Down	TG/ Gutter	TG/ Gutter						
	МН	MH	Acres	Acres	Coeff.	Coeff.	Cf*C*A	min.	(In / Hr.)	cfs	Feet	or Rise	Boxes	box %	6 n"	cfs	fps	stream	fps	%	Ft.	Stream	Stream	Elevation	Elevation	MH	flowline	MH	flowline	Top of Pipe	
					С	C*C <sub>f</sub>				Q		(ft)		(ft)		Q		manhole						Upstream	Downstream	ı UP	upstream	DOWN	downstream	(upstream)	(downstream
' A ''	1	Pond	0.15	0.15	0.80	0.80	0.12	22.16	9.06	1.09	85	12	1	0.	14 0.01	2 2.57	3.27	0.00	3.12	0.079	0.067	68.69	68.62	74.40		1 1	71.40	Pond	71.03	72.40	72.03
^		FUIU	0.13	0.13	0.00	0.00	0.12	22.10	9.00	1.09	00	12	l l	0.	14   0.01	2 2.31	3.21	0.00	3.12	0.019	0.007		*10-YR WS	4. 4. 4. 1. 4. 1. 1.	-	1 1	71.40	Folia	71.03	72.40	12.03
'A"	2	Pond	0.42	0.42	0.80	0.80	0.34	23.58	8.79	2.95	0	12	1	0.	14 0.01	2 2.57	3.27	0.00	3.76	0.585	0.000	69.68	69.68	73.40	-	2	66.74	Pond	66.74	67.74	67.74
	_		0	02	0.00	0.00	0.01	20.00	0.10	2.00							0.27	0.00	56	0.000	0.000		*10-YR WS			_					
'A"	3	Pond	0.44	0.44	0.80	0.80	0.35	23.65	8.77	3.09	0	12	1	0.	14 0.01	2 2.57	3.27	0.00	3.93	0.640	0.000	69.68	69.68	74.40	-	3	68.70	Pond	68.70	69.70	69.70
																							*10-YR WS	EL							
'A"	4	Pond	0.14	0.14	0.80	0.80	0.11	22.07	9.07	1.02	63	12	1	0.	14 0.01	2 2.57	3.27	0.00	3.05	0.069	0.044	68.66	68.62	75.50	-	4	71.97	Pond	71.69	72.97	72.69
																							*10-YR WS								
'A"_	5	Pond	0.42	0.42	0.80	0.80	0.34	23.58	8.79	2.95	0	12	1	0.	14 0.01	2 2.57	3.27	0.00	3.76	0.585	0.000	69.68	69.68	73.40	-	5	71.69	Pond	71.69	72.69	72.69
'D"	Donal	10	0.00	1 57	0.00	0.00	1.00	25.02	0.40	10 FF	60	20	4		12 0.04	0 05 40	2 5 5	0.00	2 20	0.024	0.042		*10-YR WS	EL	75.20	Dend	CC 74	10	66.67	60.74	60.67
'B"	Pond LS	LS MH1	0.00	1.57 1.57	0.80	0.80	1.26 1.26	25.83 26.11	8.40 8.36	10.55 10.55	60 32	36 24	1	0.			3.55 3.32	0.00	3.39 3.78	0.021 0.185	0.013 0.059	69.68 68.67	69.67 73.39	75.30	75.30 74.20	Pond LS	66.74 66.67	LS MH1	66.67 71.39	69.74 68.67	69.67 73.39
'D"	MH1	OUT	0.00	1.57	0.80	0.80	1.26	26.27	8.33	10.55	24	24	1	0.			3.32	0.00	3.78	0.185	0.039	73.39	73.35	74.20	74.20	MH1	71.39	OUT	71.39	73.39	73.35
	IVIIII	001	0.00	1.07	0.00	0.00	1.20	20.21	0.00	10.00		47		U.	0.01	L 10.40	0.02	0.00	0.70	0.100	U.UTT	10.00	10.00	17.20	7 7.00	IVIII	11.00	001	11.00	10.00	10.00
Ь															2,32,32,3		90) (16) (16) (16)		NC 00: 1 300								71.35			1	

# CITY OF HOUSTON RESTRICTOR CALCULATIONS

LOW LEVEL RESTRICTOR (25% FLOW)		
TOTAL DRAINAGE AREA	1.671	ACRES
OUTFLOW RATE ALLOWED FOR LOW FLOW (QL1)	0.836	CFS
HEAD HL1 FOR LOW FLOW	1.06	FT
CALCULATED LOW LEVEL RESTRICTOR SIZE DL1	6.0	INCHES
PROVIDED/DESIGNED LOW LEVEL RESTRICTOR SIZE DL1	6.0	INCHES
HIGH LEVEL RESTRICTOR (75%)		•
TOTAL DRAINAGE AREA	1.671	ACRES
TOTAL OUTFLOW RATE ALLOWED (100%) Q	3.34	CFS
RECALCULATED HEAD HL2 FOR LOW LEVEL RESTRICTOR	3.08	FT
RECALCULATED LOW FLOW QL2 FOR LOW LEVEL RESTRICTOR	2.21	CFS
OUTFLOW RATE ALLOWED FOR HIGH FLOW QH1 (75%)	N/A	CFS
HEAD Hh2 FOR HIGH LEVEL RESTRICTOR	N/A	FT
CALCULATED HIGH LEVEL RESTRICTOR Dh	N/A	INCHES
PROVIDED/DESIGNED HIGH LEVEL RESTRICTOR SIZE Dh1 (75%)	N/A	INCHES
OUTFLOW RATE PROVIDED FOR HIGH FLOW Qh2	N/A	CFS

NOTE: THE LIFT STATION WILL SERVE AS THE RESTRICTOR. (REF. SHEET C10.1 FOR DETAILS)

ORIFICE CALCULATIONS:
ALLOWABLE RUNOFF (Q) OF EACH STORM EVENT IS BASED PER COH CHAPTER 9.

 $\rm Q_{LOWER}$  = 0.5 CFS / AC; WHERE H = 25% BASIN DEPTH  $\rm Q_{UPPER}$  = 2.0 CFS -  $\rm Q_{LOWER}$  / AC; WHERE HIGH FLOW £=75% BASIN DEPTH

ORIFICE EQUATIONS:  $Q = CA\sqrt{2GH}$   $D = \sqrt{Q}$   $2.25 \text{ H}^{1/4}$  $A = \frac{Q}{C\sqrt{2GH}}$ 

WHERE: Q = ALLOWABLE OUTFLOW DISCHARGE IN (CFS)
C = DISCHARGE COEFFICIENT, 0.8 FOR SHORT PIPE;
0.6 OPENING IN PLATES, STANDPIPES, OR HEADWALLS

A = ORIFICE AREA (SF)
G = GRAVITATIONAL FACTOR (32.2 FT/S<sup>2</sup>)
H = HEAD, WATER SURFACE ELEVATION (FT)
D = ORIFICE DIAMETER (FT)

STORMWATER DETENTION VOLUME DETERMINATION								
TOTAL TRACT AREA	1.62	ACRES						
SITE IMPERVIOUS COVER	1.42	ACRES						
IMPERVIOUS COVER IN R.O.W.	0.08	ACRES						
REQUIRED DETENTION RATIO	0.95	ACRE - FEET / ACRE						
DETENTION STORAGE REQUIRED	1.43	ACRE - FEET						
TOTAL DETENTION STORAGE PROVIDED	1.66	ACRE - FEET						
REMAINING AVAILABLE DETENTION	0.23	ACRE - FEET						

REV	DESCRIPTION	DATE
	ISSUE FOR CONSTRUCTION	04-23-2024
	SESTATE OF TELL	



WARD, GETZ & ASSOCIATES, PLLC TEXAS REGISTERED ENGINEERING FIRM F-9756 2500 Tanglewilde, Suite 120 Houston, Texas 77063 713.789.1900

LOST OAKS

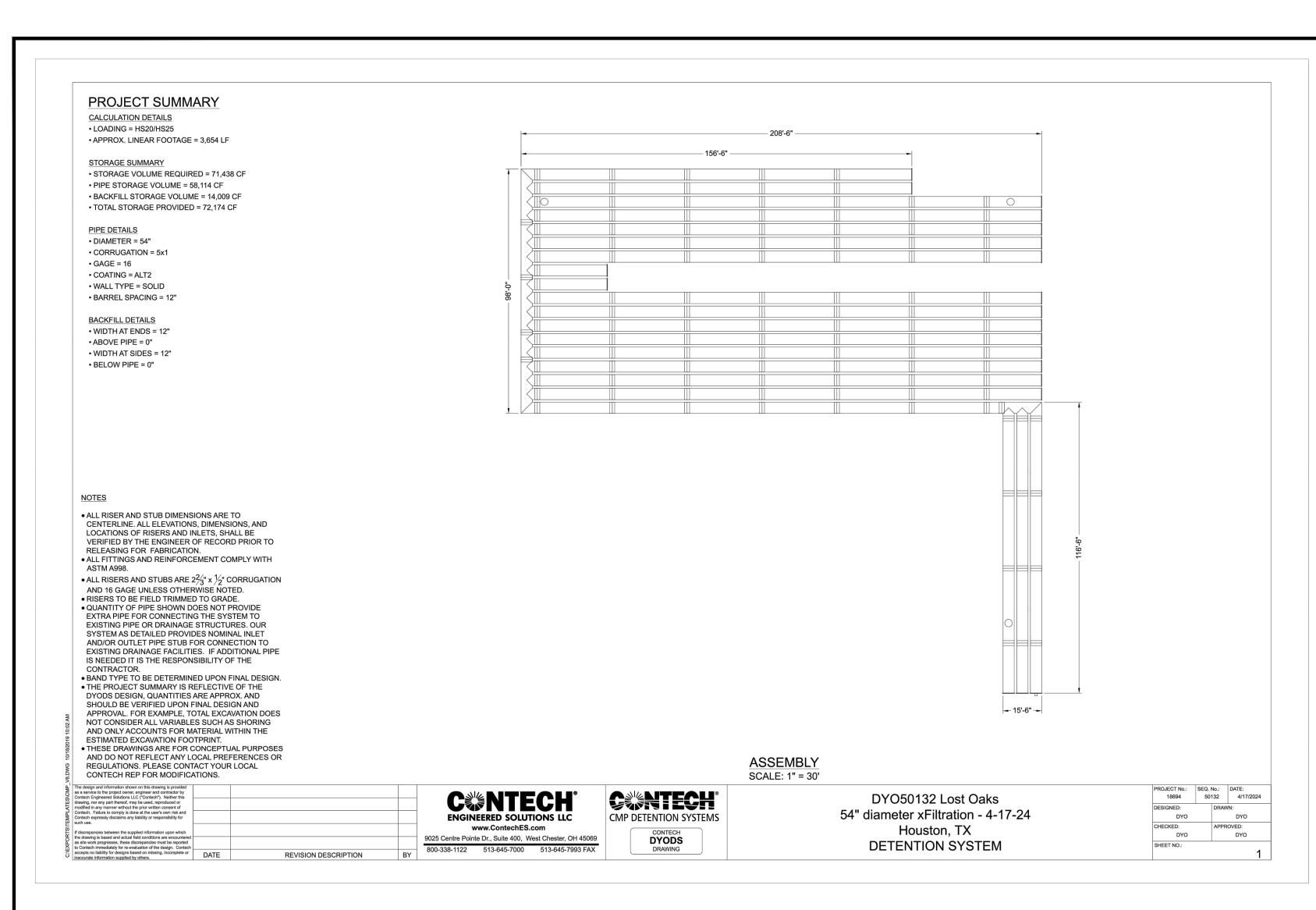
DRAINAGE AND DETENTION CALCULATIONS

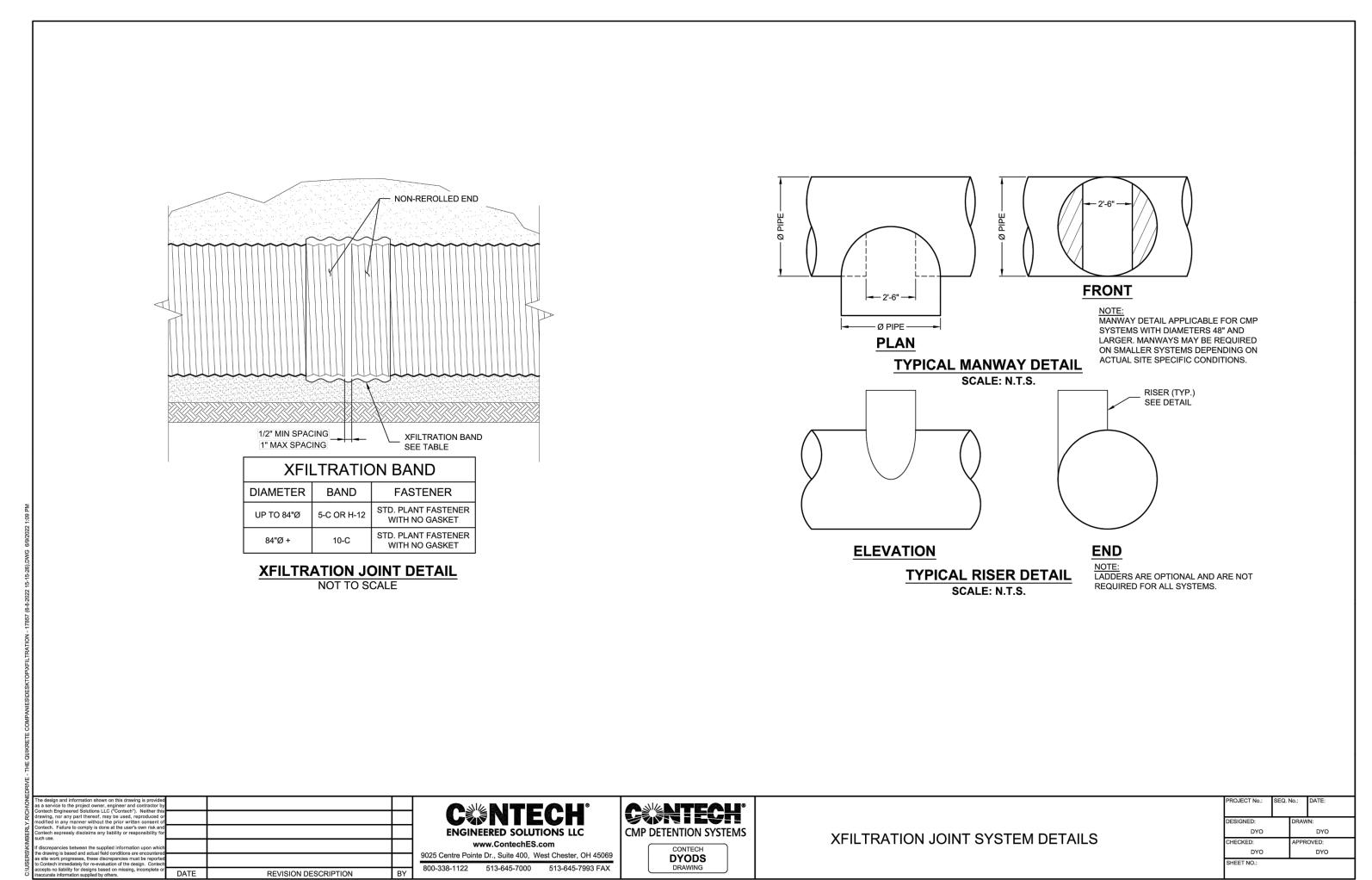
AB, AG AB, AG

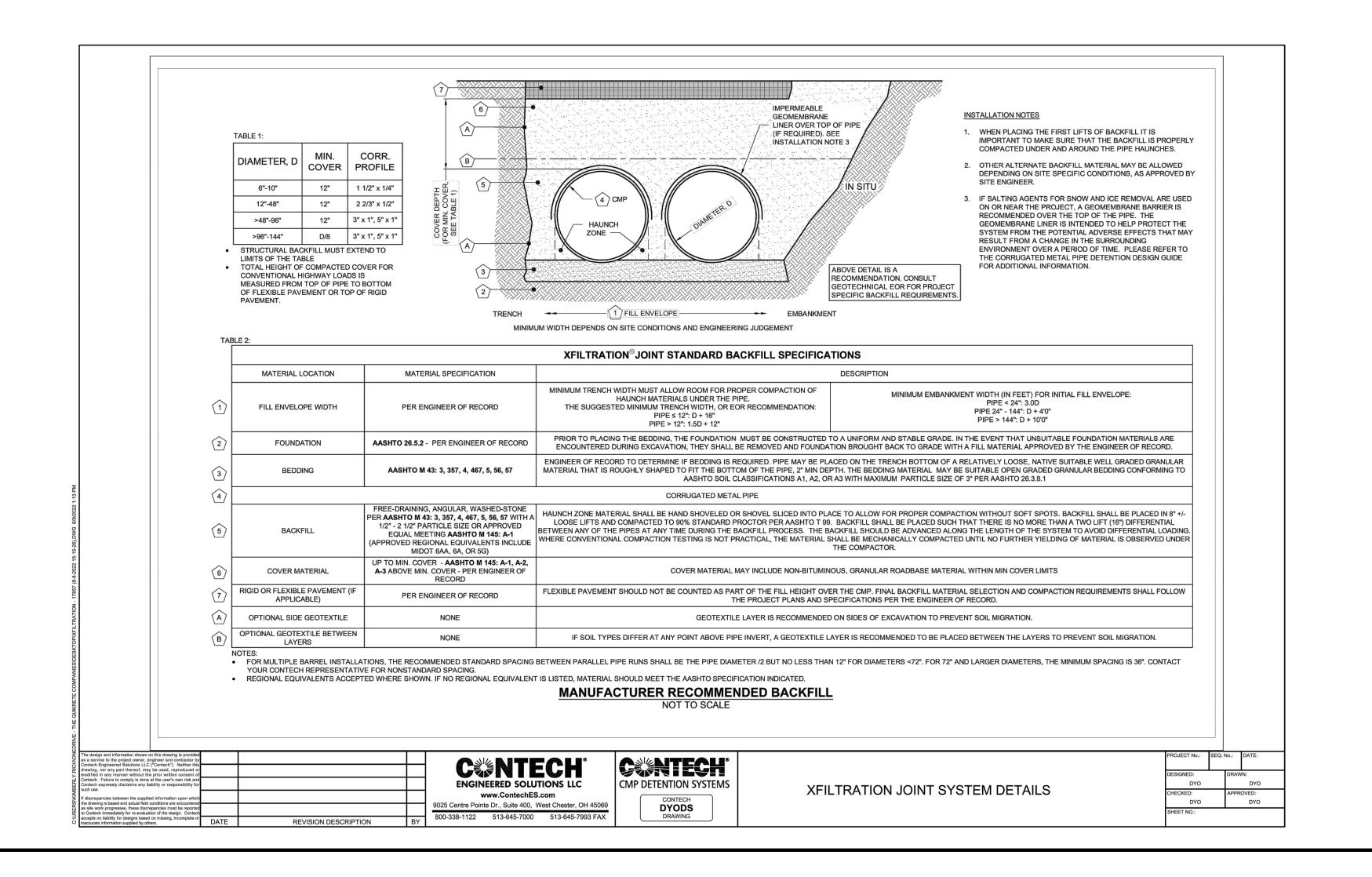
SHEET

C6.2

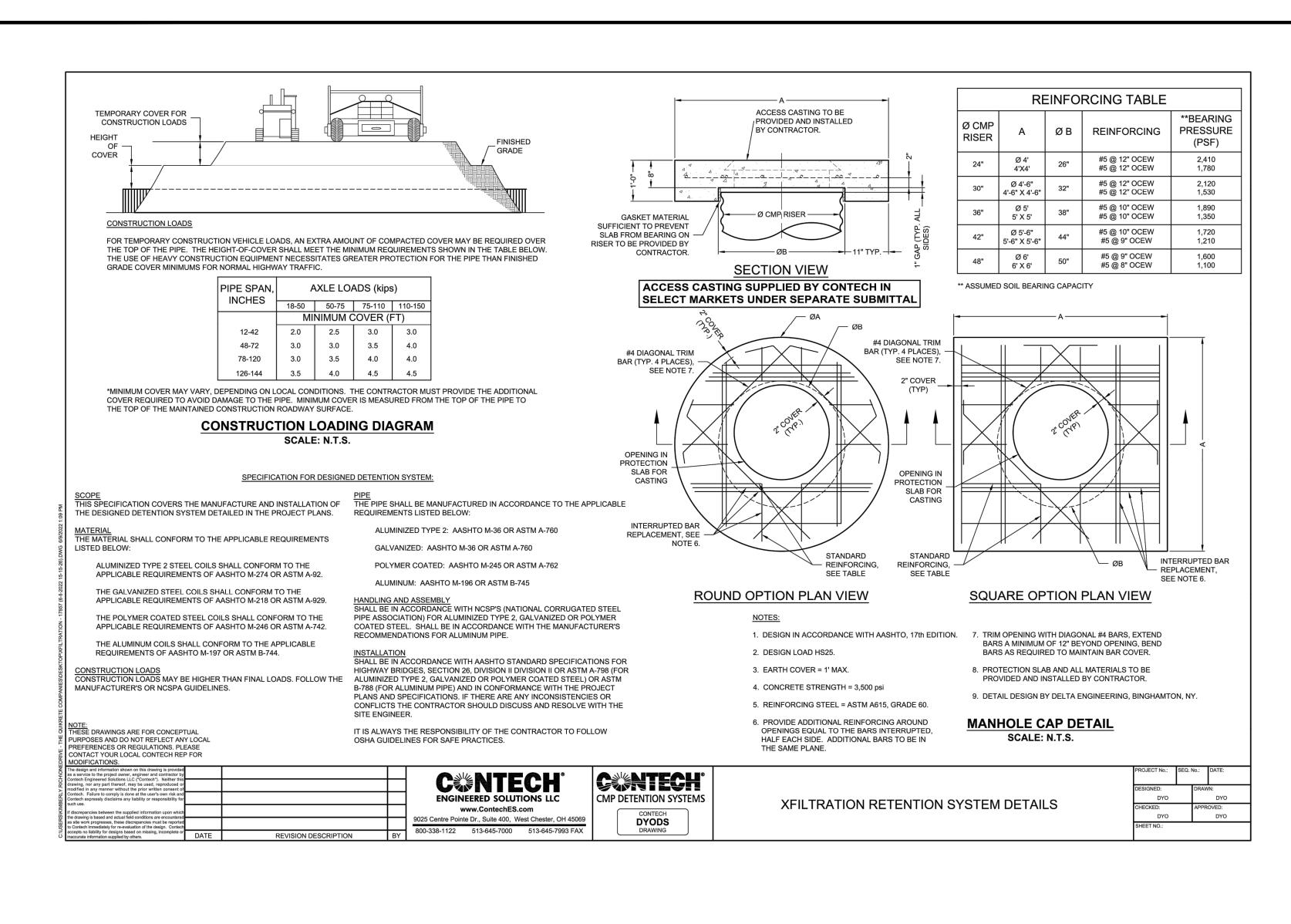
Drawing: Z:\00443 (Mark Dana)\007 Lost Oaks\CAD\C6.2 Drainage Area Calculations.dwg Last Plotted: Tue Apr 23, 2024 - 12:07pm By: ageldenhuys







DESCRIPTION

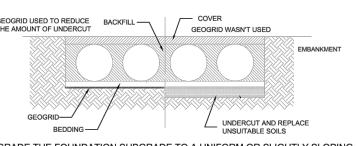




SYSTEMS OFTEN REQUIRES SPECIAL CONSTRUCTION PRACTICES THAT DIFFER FROM CONVENTIONAL FLEXIBLE PIPE CONSTRUCTION. CONTECH ENGINEERED SOLUTIONS STRONGLY SUGGESTS SCHEDULING A PRE-CONSTRUCTION MEETING WITH YOUR LOCAL SALES ENGINEER TO DETERMINE IF ADDITIONAL MEASURES, NOT COVERED IN THIS GUIDE, ARE APPROPRIATE FOR YOUR SITE.

CONSTRUCT A FOUNDATION THAT CAN SUPPORT THE DESIGN LOADING APPLIED BY THE PIPE AND ADJACENT BACKFILL WEIGHT AS WELL AS MAINTAIN ITS INTEGRITY DURING CONSTRUCTION.

IF SOFT OR UNSUITABLE SOILS ARE ENCOUNTERED, REMOVE THE POOR SOILS DOWN TO A SUITABLE DEPTH AND THEN BUILD UP TO THE APPROPRIATE ELEVATION WITH A COMPETENT BACKFILL MATERIAL. THE STRUCTURAL FILL MATERIAL GRADATION SHOULD NOT ALLOW THE MIGRATION OF FINES, WHICH CAN CAUSE SETTLEMENT OF THE DETENTION SYSTEM OR BEDDING - WELL GRADED PAVEMENT ABOVE. IF THE STRUCTURAL FILL MATERIAL IS NOT COMPATIBLE WITH THE UNDERLYING SOILS AN ENGINEERING FABRIC SHOULD BE USED AS A SEPARATOR. IN SOME CASES, USING A STIFF REINFORCING GEOGRID REDUCES OVER EXCAVATION AND REPLACEMENT FILL QUANTITIES.

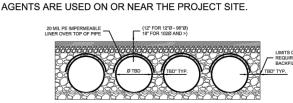


GRADE THE FOUNDATION SUBGRADE TO A UNIFORM OR SLIGHTLY SLOPING GRADE. IF THE SUBGRADE IS CLAY OR RELATIVELY NON-POROUS AND THE CONSTRUCTION SEQUENCE WILL LAST FOR AN EXTENDED PERIOD OF TIME IT IS BEST TO SLOPE THE GRADE TO ONE END OF THE SYSTEM. THIS WILL ALLOW EXCESS WATER TO DRAIN QUICKLY, PREVENTING SATURATION OF THE SUBGRADE.

#### GEOMEMBRANE BARRIER

A SITE'S RESISTIVITY MAY CHANGE OVER TIME WHEN VARIOUS TYPES OF SALTING AGENTS ARE USED, SUCH AS ROAD SALTS FOR DEICING AGENTS. IF SALTING AGENTS ARE USED ON OR NEAR THE PROJECT SITE. A GEOMEMBRANE BARRIER IS RECOMMENDED WITH THE SYSTEM. THE GEOMEMBRANE LINER IS INTENDED TO HELP PROTECT THE SYSTEM FROM THE TO THE END OF THE RECENTLY PLACED FILL, AND BEGIN THE SEQUENCE POTENTIAL ADVERSE EFFECTS THAT MAY RESULT FROM THE USE OF SUCH

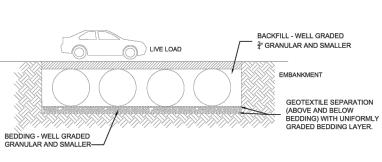
THE PROJECT'S ENGINEER OF RECORD IS TO EVALUATE WHETHER SALTING AGENTS WILL BE USED ON OR NEAR THE PROJECT SITE, AND USE HIS/HER BEST JUDGEMENT TO DETERMINE IF ANY ADDITIONAL PROTECTIVE MEASURES ARE REQUIRED. BELOW IS A TYPICAL DETAIL SHOWING THE PLACEMENT OF A GEOMEMBRANE BARRIER FOR PROJECTS WHERE SALTING



#### **IN-SITU TRENCH WALL**

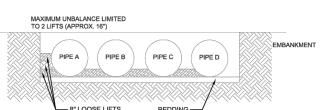
PROPER INSTALLATION OF A FLEXIBLE UNDERGROUND DETENTION SYSTEM IF EXCAVATION IS REQUIRED, THE TRENCH WALL NEEDS TO BE CAPABLE OF WILL ENSURE LONG-TERM PERFORMANCE. THE CONFIGURATION OF THESE SUPPORTING THE LOAD THAT THE PIPE SHEDS AS THE SYSTEM IS LOADED. IF SOILS ARE NOT CAPABLE OF SUPPORTING THESE LOADS, THE PIPE CAN DEFLECT. PERFORM A SIMPLE SOIL PRESSURE CHECK USING THE APPLIED LOADS TO DETERMINE THE LIMITS OF EXCAVATION BEYOND THE SPRING LINE OF BALANCE BETWEEN THE UPLIFT FORCE OF THE CLSM, THE OPPOSING THE OUTER MOST PIPES.

IN MOST CASES THE REQUIREMENTS FOR A SAFE WORK ENVIRONMENT AND PROPER BACKFILL PLACEMENT AND COMPACTION TAKE CARE OF THIS CONCERN. LIFT THICKNESS. YOUR LOCAL CONTECH SALES ENGINEER CAN HELP



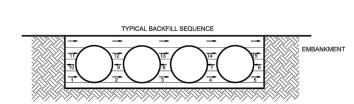
#### BACKFILL PLACEMENT

MATERIAL SHALL BE WORKED INTO THE PIPE HAUNCHES BY MEANS OF SHOVEL-SLICING, RODDING, AIR TAMPER, VIBRATORY ROD, OR OTHER EFFECTIVE METHODS.



IF AASHTO T99 PROCEDURES ARE DETERMINED INFEASIBLE BY THE GEOTECHNICAL ENGINEER OF RECORD, COMPACTION IS CONSIDERED ADEQUATE WHEN NO FURTHER YIELDING OF THE MATERIAL IS OBSERVED UNDER THE COMPACTOR, OR UNDER FOOT, AND THE GEOTECHNICAL ENGINEER OF RECORD (OR REPRESENTATIVE THEREOF) IS SATISFIED WITH THE LEVEL OF COMPACTION.

FOR LARGE SYSTEMS, CONVEYOR SYSTEMS, BACKHOES WITH LONG REACHES OR DRAGLINES WITH STONE BUCKETS MAY BE USED TO PLACE BACKFILL. ONCE MINIMUM COVER FOR CONSTRUCTION LOADING ACROSS THE ENTIRE WIDTH OF THE SYSTEM IS REACHED, ADVANCE THE EQUIPMENT AGAIN UNTIL THE SYSTEM IS COMPLETELY BACKFILLED. THIS TYPE OF AGENTS INCLUDING PREMATURE CORROSION AND REDUCED ACTUAL SERVICE CONSTRUCTION SEQUENCE PROVIDES ROOM FOR STOCKPILED BACKFILL DIRECTLY BEHIND THE BACKHOE, AS WELL AS THE MOVEMENT OF CONSTRUCTION TRAFFIC. MATERIAL STOCKPILES ON TOP OF THE BACKFILLED DETENTION SYSTEM SHOULD BE LIMITED TO 8- TO 10-FEET HIGH AND MUST PROVIDE BALANCED LOADING ACROSS ALL BARRELS. TO DETERMINE THE PROPER COVER OVER THE PIPES TO ALLOW THE MOVEMENT OF CONSTRUCTION EQUIPMENT SEE TABLE 1, OR CONTACT YOUR LOCAL CONTECH SALES ENGINEER.



#### CMP DETENTION SYSTEM INSPECTION AND MAINTENANCE

UNDERGROUND STORMWATER DETENTION AND INFILTRATION SYSTEMS MUST BE INSPECTED AND MAINTAINED AT REGULAR INTERVALS FOR PURPOSES OF PERFORMANCE AND LONGEVITY.

#### INSPECTION

NSPECTION IS THE KEY TO EFFECTIVE MAINTENANCE OF CMP DETENTION SYSTEMS AND IS EASILY PERFORMED. CONTECH RECOMMENDS ONGOING, ANNUAL INSPECTIONS. SITES WITH HIGH TRASH LOAD OR SMALL OUTLET CONTROL ORIFICES MAY NEED MORE FREQUENT INSPECTIONS. THE RATE AT WHICH THE SYSTEM COLLECTS POLLUTANTS WILL DEPEND MORE ON SITE SPECIFIC ACTIVITIES RATHER THAN THE SIZE OR CONFIGURATION OF THE

INSPECTIONS SHOULD BE PERFORMED MORE OFTEN IN EQUIPMENT WASHDOWN AREAS, IN CLIMATES WHERE SANDING AND/OR SALTING OPERATIONS TAKE PLACE, AND IN OTHER VARIOUS INSTANCES IN WHICH ONE WOULD EXPECT HIGHER ACCUMULATIONS OF SEDIMENT OR ABRASIVE/ CORROSIVE CONDITIONS. A RECORD OF EACH INSPECTION IS TO BE MAINTAINED FOR THE LIFE OF THE SYSTEM

#### MAINTENANCE

CMP DETENTION SYSTEMS SHOULD BE CLEANED WHEN AN INSPECTION REVEALS ACCUMULATED SEDIMENT OR TRASH IS CLOGGING THE DISCHARGE

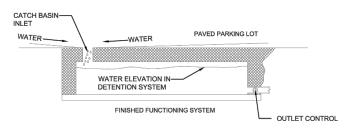
ACCUMULATED SEDIMENT AND TRASH CAN TYPICALLY BE EVACUATED THROUGH THE MANHOLE OVER THE OUTLET ORIFICE. IF MAINTENANCE IS NOT PERFORMED AS RECOMMENDED, SEDIMENT AND TRASH MAY ACCUMULATE IN FRONT OF THE OUTLET ORIFICE. MANHOLE COVERS SHOULD BE SECURELY SEATED FOLLOWING CLEANING ACTIVITIES. CONTECH SUGGESTS THAT ALL SYSTEMS BE DESIGNED WITH AN ACCESS/INSPECTION MANHOLE SITUATED AT OR NEAR THE INLET AND THE OUTLET ORIFICE. SHOULD IT BE NECESSARY TO GET INSIDE THE SYSTEM TO PERFORM MAINTENANCE ACTIVITIES, ALL APPROPRIATE PRECAUTIONS REGARDING CONFINED SPACE ENTRY AND OSHA REGULATIONS SHOULD BE FOLLOWED.

ANNUAL INSPECTIONS ARE BEST PRACTICE FOR ALL UNDERGROUND SYSTEMS. DURING THIS INSPECTION, IF EVIDENCE OF SALTING/DE-ICING AGENTS IS OBSERVED WITHIN THE SYSTEM, IT IS BEST PRACTICE FOR THE SYSTEM TO BE RINSED, INCLUDING ABOVE THE SPRING LINE SOON AFTER THE

SPRING THAW AS PART OF THE MAINTENANCE PROGRAM FOR THE SYSTEM. MAINTAINING AN UNDERGROUND DETENTION OR INFILTRATION SYSTEM IS

EASIEST WHEN THERE IS NO FLOW ENTERING THE SYSTEM. FOR THIS REASON, IT IS A GOOD IDEA TO SCHEDULE THE CLEANOUT DURING DRY

MAY BE REQUIRED FOR HIGH FLOWS DUE TO THE RESTRICTED NATURE OF THE FOREGOING INSPECTION AND MAINTENANCE EFFORTS HELP ENSURE UNDERGROUND PIPE SYSTEMS USED FOR STORMWATER STORAGE CONTINUE TO FUNCTION AS INTENDED BY IDENTIFYING RECOMMENDED REGULAR INSPECTION AND MAINTENANCE PRACTICES. INSPECTION AND MAINTENANCE RELATED TO THE STRUCTURAL INTEGRITY OF THE PIPE OR THE SOUNDNESS OF PIPE JOINT CONNECTIONS IS BEYOND THE SCOPE OF THIS GUIDE.



WHEN FLOWABLE FILL IS USED, YOU MUST PREVENT PIPE FLOATATION.

TYPICALLY, SMALL LIFTS ARE PLACED BETWEEN THE PIPES AND THEN

ALLOWED TO SET-UP PRIOR TO THE PLACEMENT OF THE NEXT LIFT. THE

ALLOWABLE THICKNESS OF THE CLSM LIFT IS A FUNCTION OF A PROPER

MEASURES. THE PIPE CAN CARRY LIMITED FLUID PRESSURE WITHOUT

PIPE DISTORTION OR DISPLACEMENT, WHICH ALSO AFFECTS THE CLSM

WEIGHT OF THE PIPE, AND THE EFFECT OF OTHER RESTRAINING

WEIGHTED PIPE WITH MOBILE

(OR OTHER REMOVABLE WEIGHTS)

TYPICALLY, THE MINIMUM COVER SPECIFIED FOR A PROJECT ASSUMES H-20

LIVE LOAD. BECAUSE CONSTRUCTION LOADS OFTEN EXCEED DESIGN LIVE

NECESSARY. SINCE CONSTRUCTION EQUIPMENT VARIES FROM JOB TO JOB,

LOADS, INCREASED TEMPORARY MINIMUM COVER REQUIREMENTS ARE

REQUIREMENTS WITH YOUR LOCAL CONTECH SALES ENGINEER DURING

BECAUSE MOST SYSTEMS ARE CONSTRUCTED BELOW-GRADE, RAINFALI

CAN RAPIDLY FILL THE EXCAVATION; POTENTIALLY CAUSING FLOATATION

AND MOVEMENT OF THE PREVIOUSLY PLACED PIPES. TO HELP MITIGATE

DOWNSTREAM END WITH THE OUTLET ALREADY CONSTRUCTED TO ALLOW

A ROUTE FOR THE WATER TO ESCAPE. TEMPORARY DIVERSION MEASURES

POTENTIAL PROBLEMS, IT IS BEST TO START THE INSTALLATION AT THE

IT IS BEST TO ADDRESS EQUIPMENT SPECIFIC MINIMUM COVER

DETERMINE THE PROPER LIFT THICKNESS.

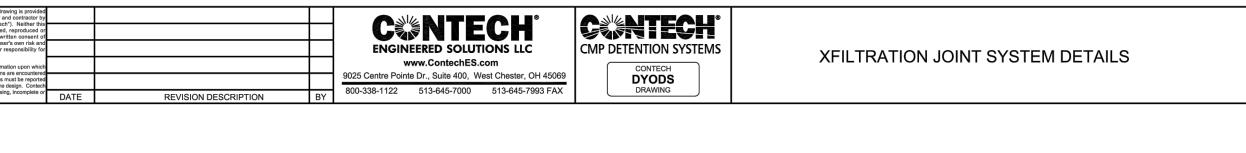
CONSTRUCTION LOADING

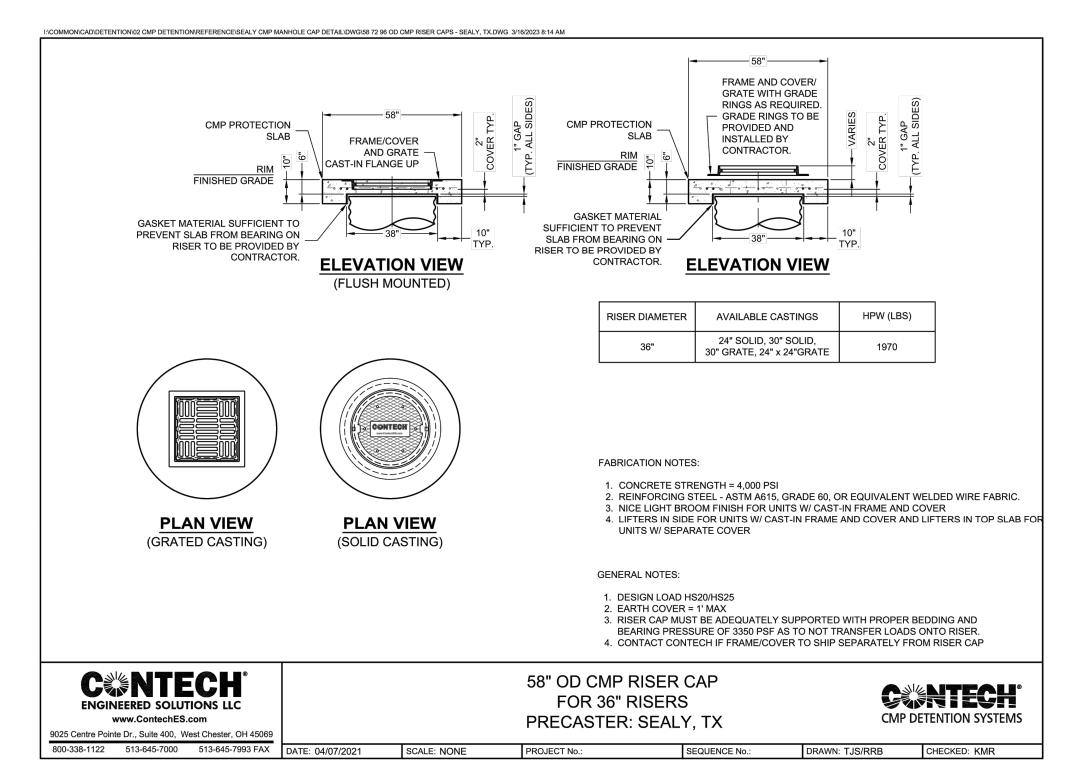
YOUR PRE-CONSTRUCTION MEETING.

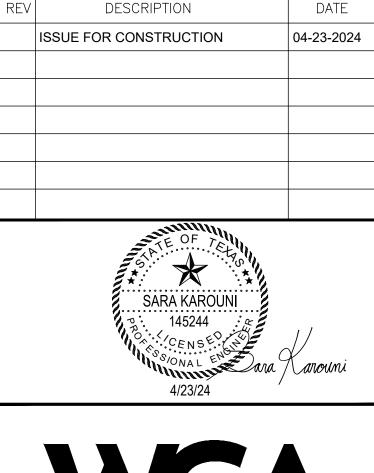
ADDITIONAL CONSIDERATIONS

THE OUTLET PIPE.

STAGE POURS AS REQUIRED TO CONTROL FLOATATION AND PIPE DISTORTION/DISPLACEMENT







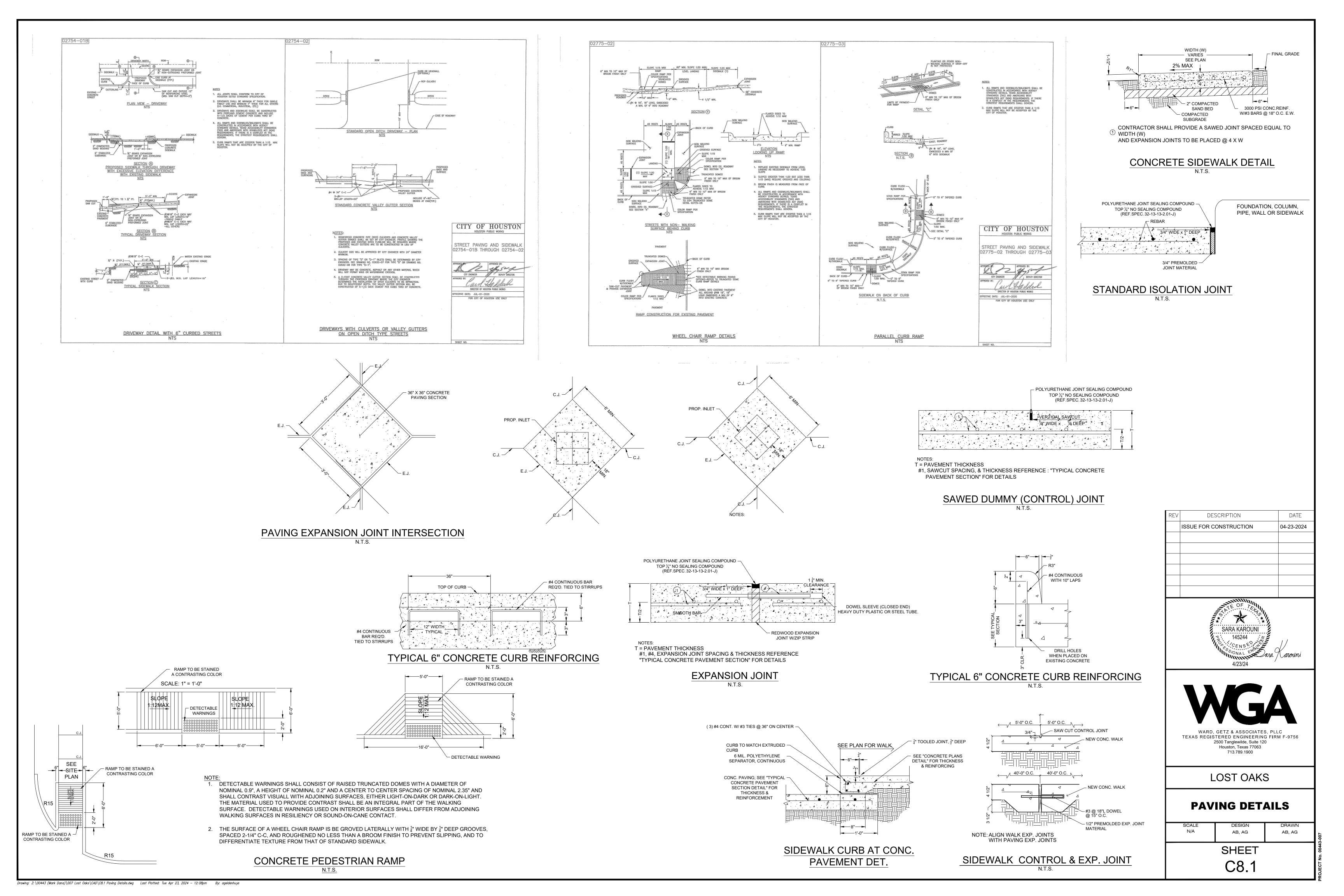


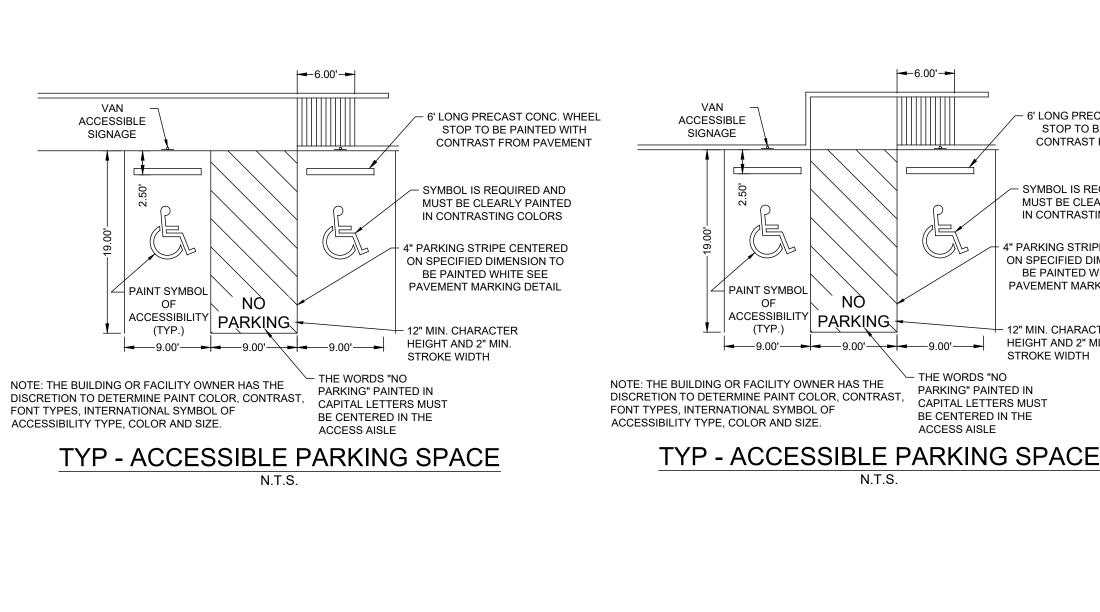
LOST OAKS

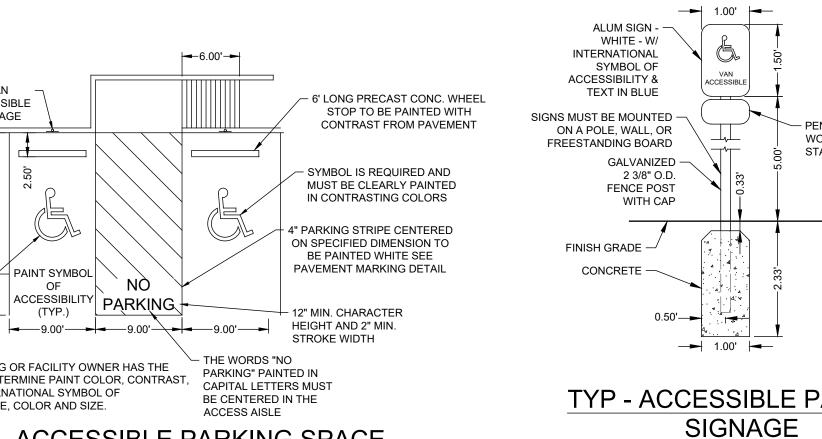
UNDERGROUND **DETENTION DETAILS** 

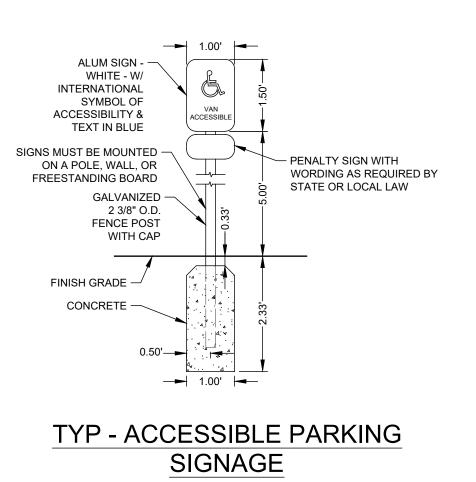
N/A AB, AG AB, AG

SHEET

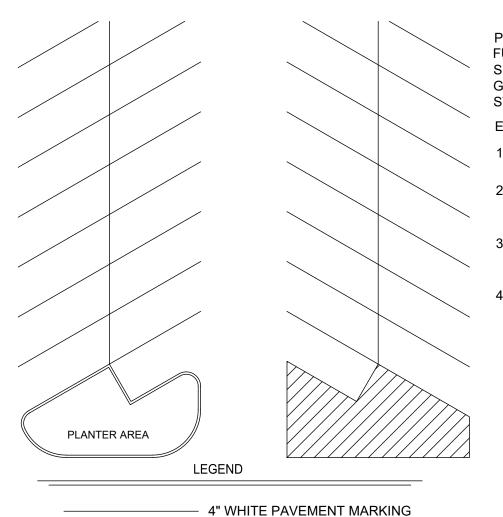








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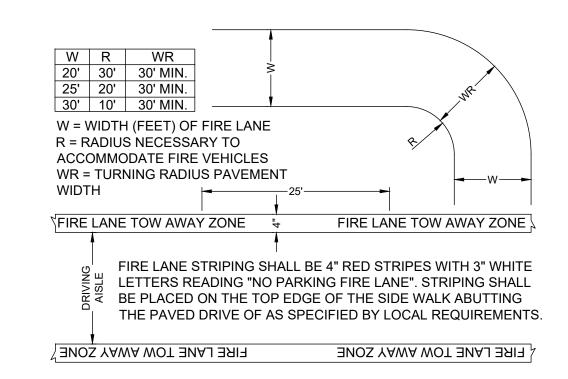


PAVEMENT MARKINGS NOTES: FURNISH AND INSTALL PAVEMENT MARKINGS OF THE TYPE AND SIZE SHOWN ON THE PLANS AND AS REQUIRED FOR COMPLIANCE WITH GOVERNING CODES. IF NO GOVERNING CODES APPLY, THEN USE TXDOT STANDARDS.

#### EXECUTION:

- MINIMUM LINE WIDTH IS 4 INCHES. PAVEMENT MARKINGS MUST COMPLY WITH LOCAL FIRE STANDARDS AND CURRENT ACCESSIBILITY CODE.
- A MINIMUM OF TWO COATS SHALL BE REQUIRED. WAIT 30 DAYS AFTER PAVEMENT INSTALLATION BEFORE APPLYING THE SECOND COAT OF PAVEMENT MARKINGS.
- 3. CLOSE AREAS TO TRAFFIC FOR DURATION OF DRYING TIME, WHICH SHALL BE NO LESS THAN THE MINIMUM RECOMMENDED BY THE PAINT MANUFACTURER.
- TRAFFIC PAINT SHALL BE AS FOLLOWS OR APPROVED EQUAL:
- CONCRETE- SHERWIN WILLIAMS, SETFAST ACRYLIC ZONE MARKING
- CONCRETE & BLUE HANDICAP BACKGROUNDS-SHERWIN WILLIAMS, SETFAST CHLORINATED
- RUBBER ZONE MARKING PAINT
- NEW ASPHALT- SHERWIN WILLIAMS, SETFAST, ACRYLIC LATEX TRAFFIC MARKING PAINT
- OLD ASPHALT- SHERWIN WILLIAMS,
- SETFAST ACRYLIC WB TRAFFIC MARKING PAINT

# PAVEMENT MARKING DETAIL



PAVEMENT

REINFORCEMENT

#3 @ STURRUPS @18" C-C

T = PAVEMENT THICKNESS

#1, SAWCUT SPACING, & THICKNESS REFERENCE: "TYPICAL

1) CONSTRUCT PAVERS/TERRAZZO PAVING AS SHOWN ON THIS SHEET.

3) THIS BLOCKOUT DIMENSION FOR PAVERS/TERRAZZO IS NOMINAL. THE CONTRACTOR MAY ADJUST BLOCKOUT AS NEEDED TO MINIMIZE THE AMOUNT OF PAVERS/TERRAZZO TO BE CUT. HOWEVER, THE PAVERS MUST ALWAYS BE HELD ONE FOOT AWAY FROM FACE OF CURB AT THE

CONCRETE PAVEMENT SECTION" FOR DETAILS

TYPICAL CURB WITH TURNDOWN

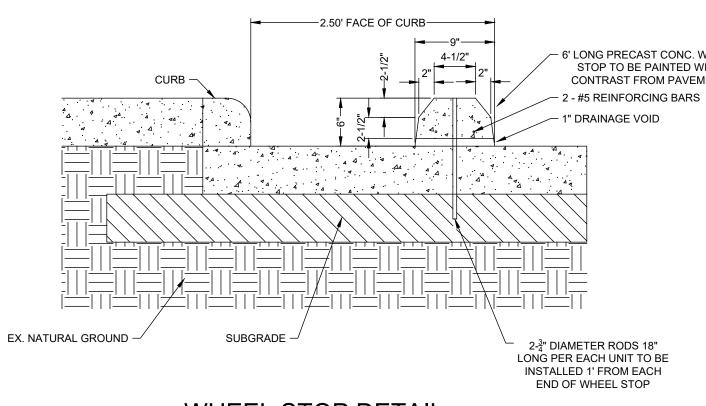
W/12"MIN. OVERLAP

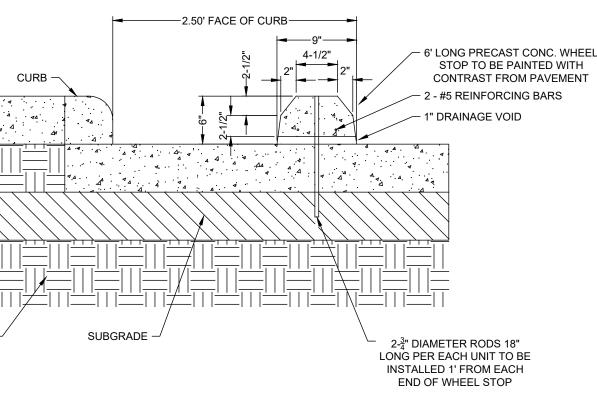
PAVER NOTES:

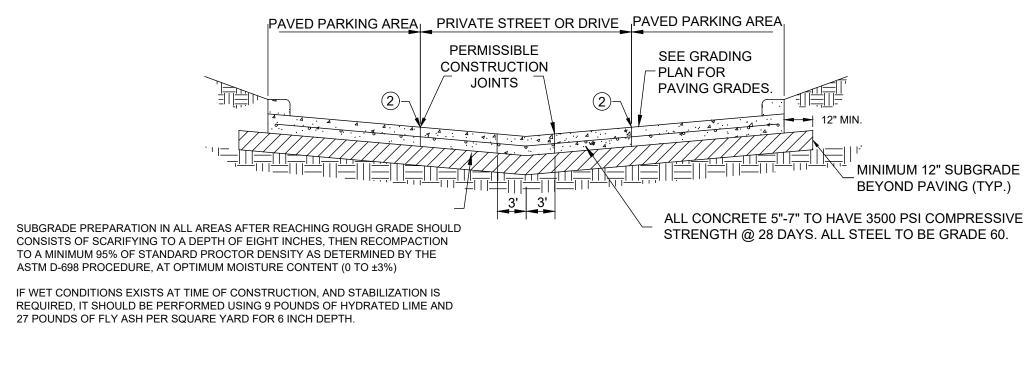
FIRE STRIPING DETAIL

\_\_ (2) #3 (TOP & BOTTOM)

2) PAVESTONE AND TERRAZZO PAVING SHALL COMPLY WITH SPECIFICATIONS OF THE MANUFACTURER.







1. REINFORCING STEEL: 5" THICK CONCRETE PAVEMENT TO HAVE #4 BARS SPACED AT 24" ON CENTER

6" THICK CONCRETE PAVEMENT TO HAVE #4 BARS SPACED AT 18" ON CENTER 7" THICK CONCRETE PAVEMENT TO HAVE #4 BARS SPACED AT

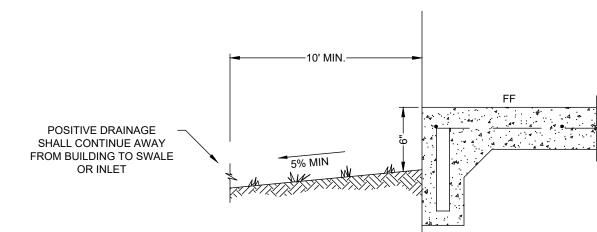
18" INCHES ON CENTER 2. \*CONTROL JOINT SPACING: 5" PAVING MAXIMUM OF 12.5' 6" & 7" PAVING MAXIMUM OF 15'

3. EXPANSION JOINT: THE INSTALLATION OF EXPANSION JOINTS IS OPTIONAL, BUT IF USED, THEY SHOULD HAVE A MAXIMUM SPACING OF 60 FEET  $3\!\!\!\!/$ " DIAMETER. 18 IN LONG WITH ONE END TREATED TO SLIP, AND 4. DOWELS AT EXPANSION JOINTS: SPACED AT 12 INCHES ON CENTER AT EACH JOINT

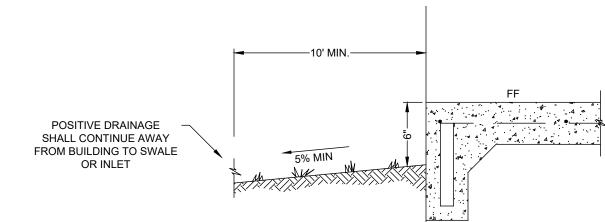
\*CONTROL JOINTS CUT WITHIN 6-12 HOURS OF CONCRETE PLACEMENT

# TYPICAL CONCRETE PAVEMENT SECTION

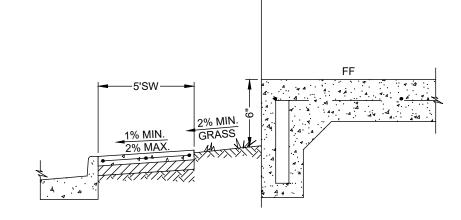
SEE GEOTECHNICAL ENGINEERING REPORT BY THE MURILLO COMPANY (PROJECT No. GEO3082023, DATED AUGUST, 2023)



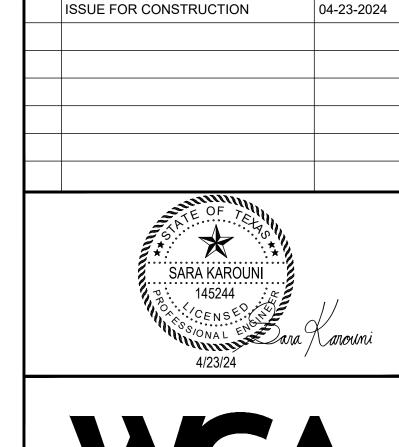
TYPICAL PERIMITER GRADES AROUND BUILDING



TYPICAL PERIMITER GRADES **AROUND BUILDING** 



TYPICAL PERIMITER GRADES **AROUND BUILDING** 



DESCRIPTION

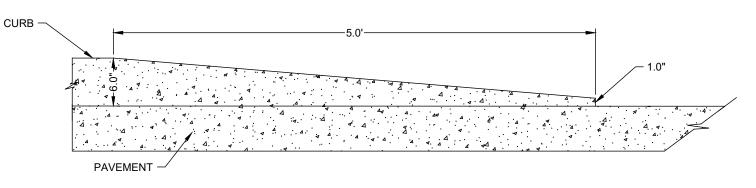


LOST OAKS

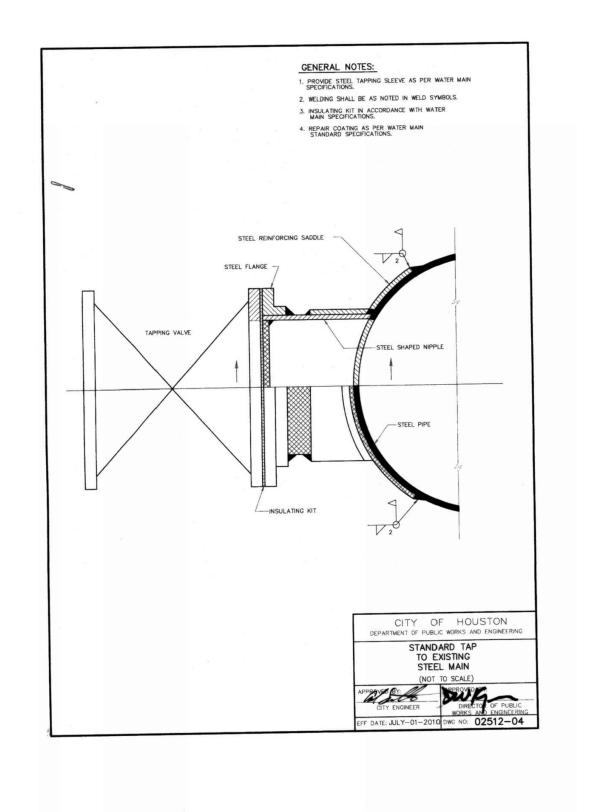
# **PAVING DETAILS**

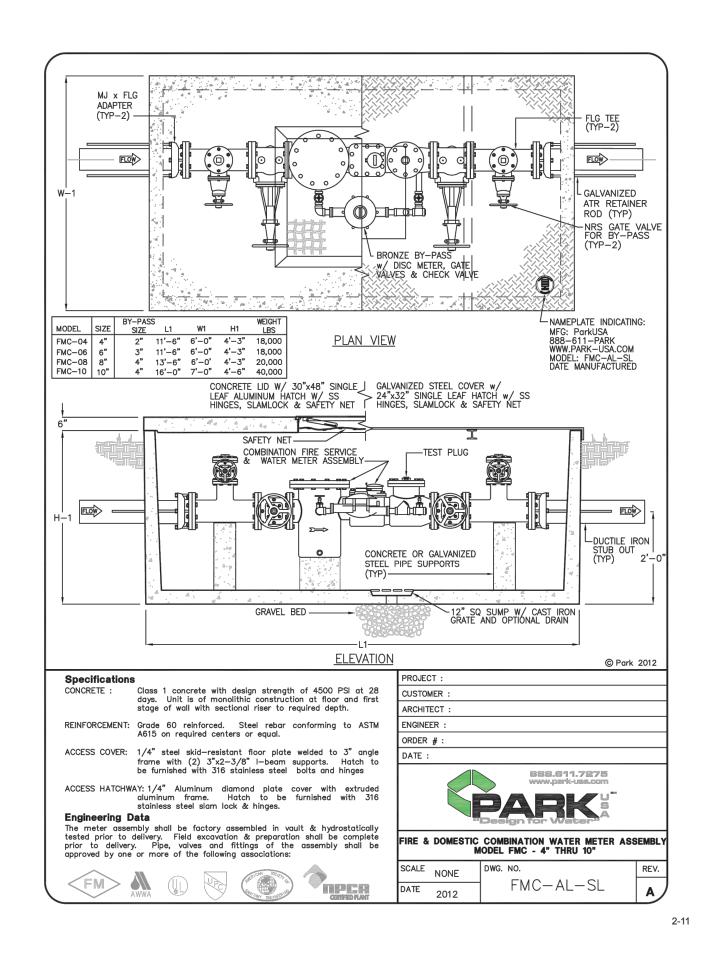
	CLIEET		Ž
N/A	AB, AG	AB, AG	3-007
SCALE	DESIGN	DRAWN	1

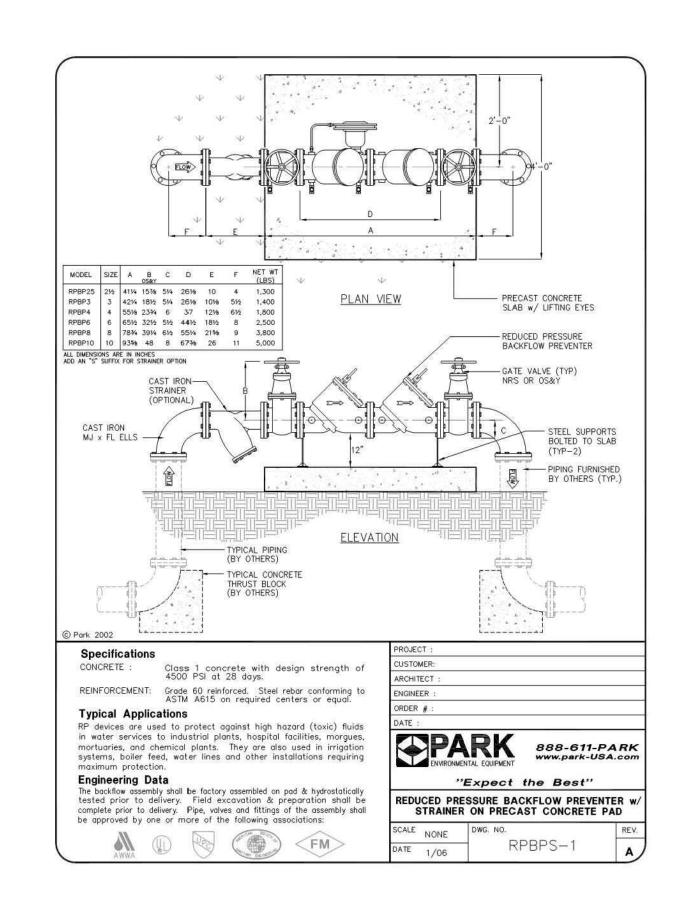
SHEET C8.2

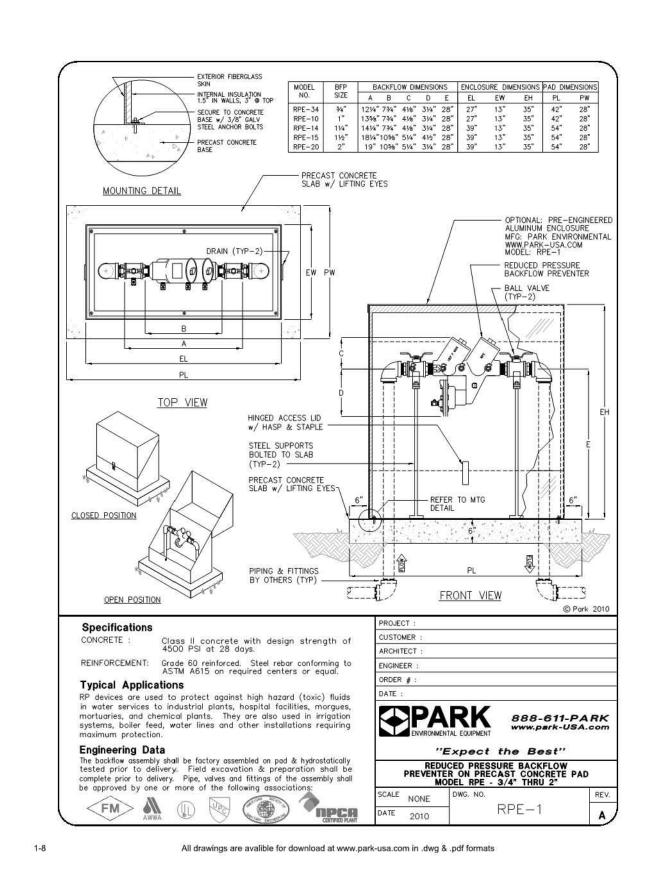


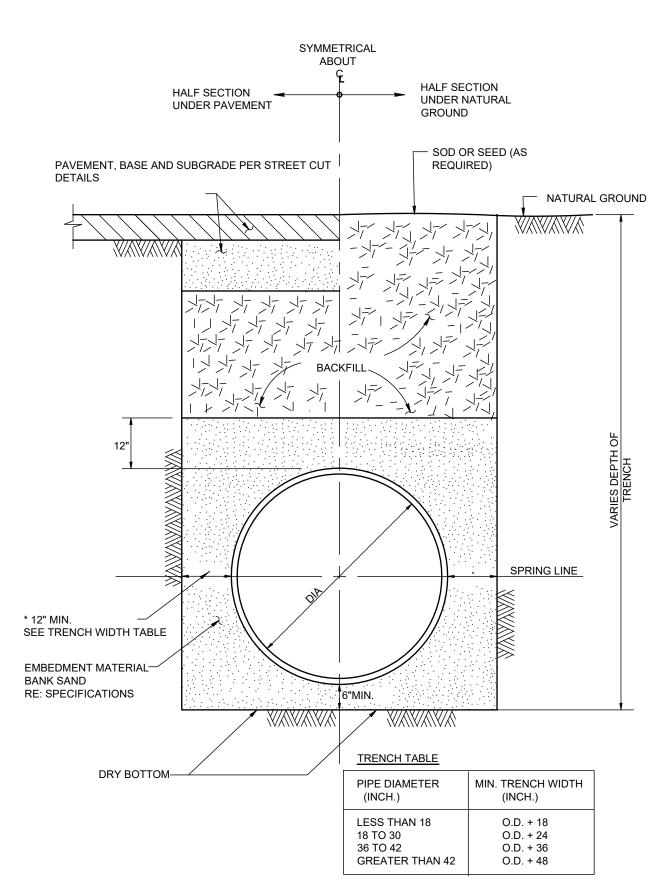
**CURB TRANSITION** 



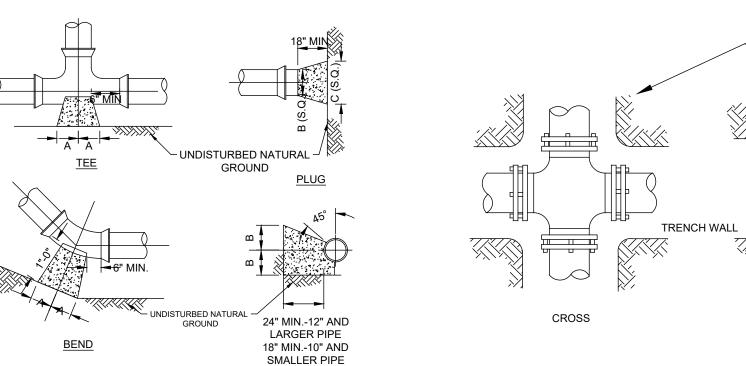




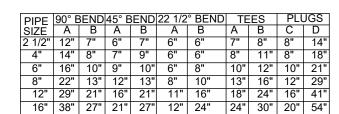




WATER DISTRIBUTION MAIN
BEDDING AND BACKFILL FOR
OPEN CUT TRENCHES
(NOT TO SCALE)



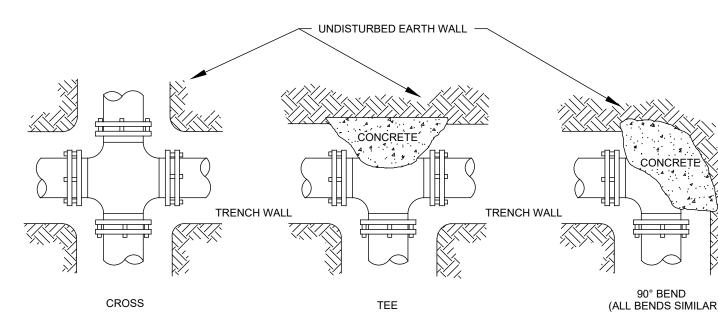
BEND & TEE

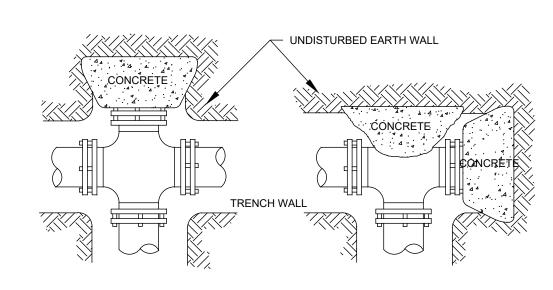


NOTE:

1. THRUST BLOCKS AT TRENCH FACE MUST HAVE A MINIMUM BEARING SURFACE OF 1.0 SQ. FOOT AND THE LEAST DIMENSION SHALL BE NO SMALLER THAN 1.5 TIMES PIPE DIAMETER, BUT NOT LESS THAN 1.0 FEET.

2. ALL CONCRETE TO BE POURED AGAINST FIRM, UNDISTURBED SOIL AND SHALL BE MIN. 5SK, 3000 P.S.I.





CROSS WITH PLUG

THRUST BLOCK DETAIL

N.T.S.

	ı	HORIZONTAL THI	RUST BLOCKING									
	(BLOCKING HEIGHT GREATER THAN PIPE O.D.) (BLOCKING WIDTH BETWEEN 1 & 2 TIMES HEIGHT)											
REQUIRED SQ. FT. OF UNDISTURBED EARTH WALL FOR REACTION BACKING												
TYPE OF FITTINGS												
PIPE	TEE &	BENDS										
SIZE	PLUG	90°	45°	22 1/2°								
3"	0.5	1.5	1.5	0.5								
4"	1.5	2.0	1.5	1.0								
6"	3.0	4.0	2.0	1.0								
8"	5.0	7.0	4.0	2.0								
10"	8.0	10.0	6.0	3.0								
12"	11.0	17.0	9.0	5.0								

DO NOT COVER BELLS OR FLANGES WITH CONCRETE
 WRAP ALL FITTINGS WITH VISQUEEN.

NOTES:

 BACK ALL TEES ACCORDING TO SIZE OF BRANCH.
 BACKING FUTURE LINE EXTENSIONS SHALL BE SUCH THAT LATER REMOVAL IS POSSIBLE.

5. ALL BENDS WHERE FITTINGS ARE USED, BOTH HORIZONTAL OR VERTICAL SHALL BE BACKED.
 6. PEACTION BACKING TABLE IS BASED ON 60 PEAL AND SOIL

6. REACTION BACKING TABLE IS BASED ON 60 P.S.I. AND SOIL BEARING PRESSURE OF 1500 lb./sq.ft. ADDITIONAL BACKING MAY BE REQUIRED IN SOME AREAS AS DIRECTED BY ENGINEERS.

7. ALL CONCRETE SHALL BE 2500 P.S.I.

8. 18" AND LARGER REQUIRES SPECIFIC ANTI-THRUST DESIGN.



DESCRIPTION

04-23-2024

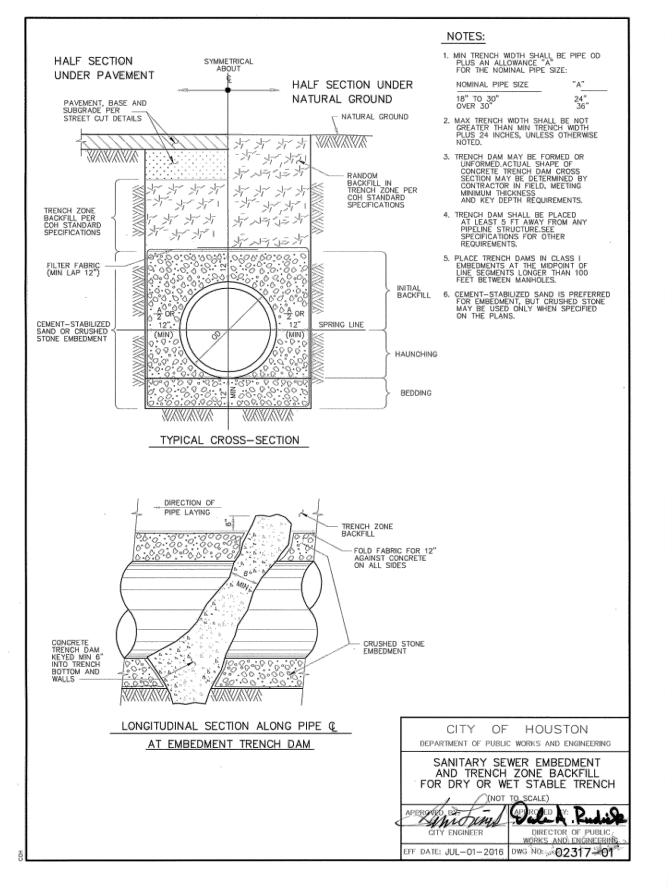
ISSUE FOR CONSTRUCTION

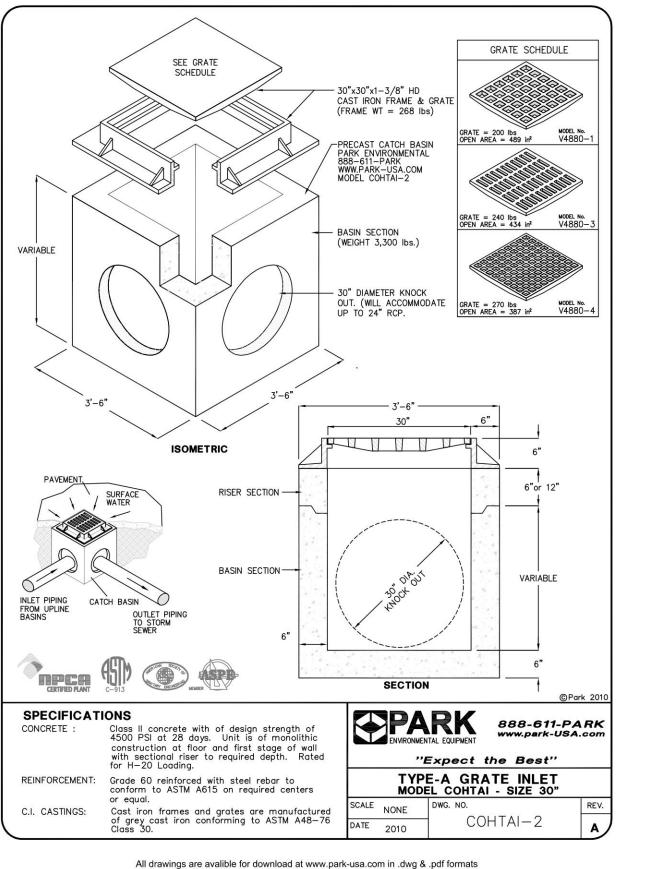
WATER	<b>DETAILS</b>

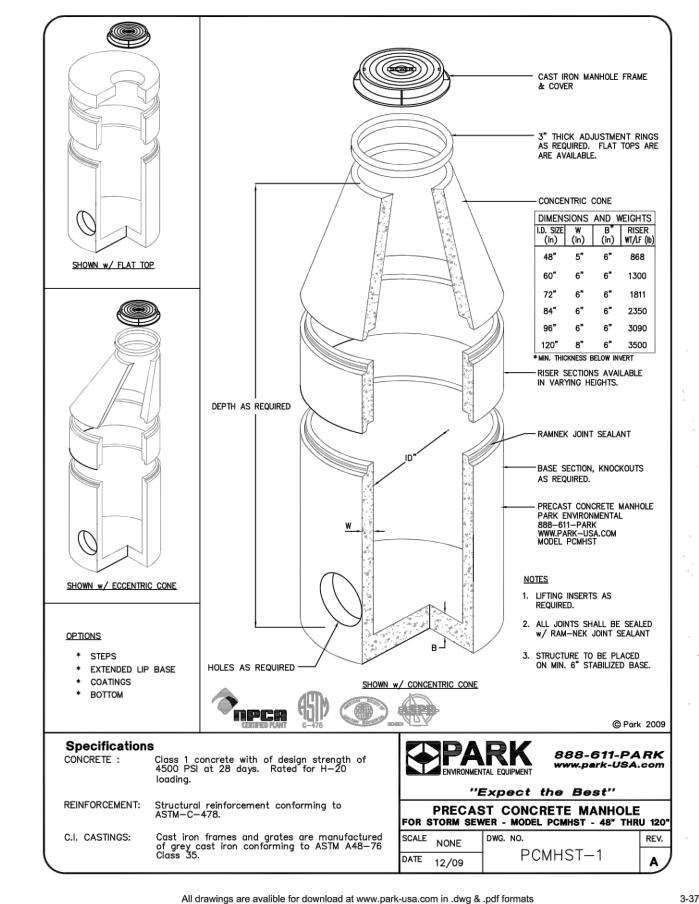
SCALE DESIGN DRAWN
N/A AB, AG AB, AG

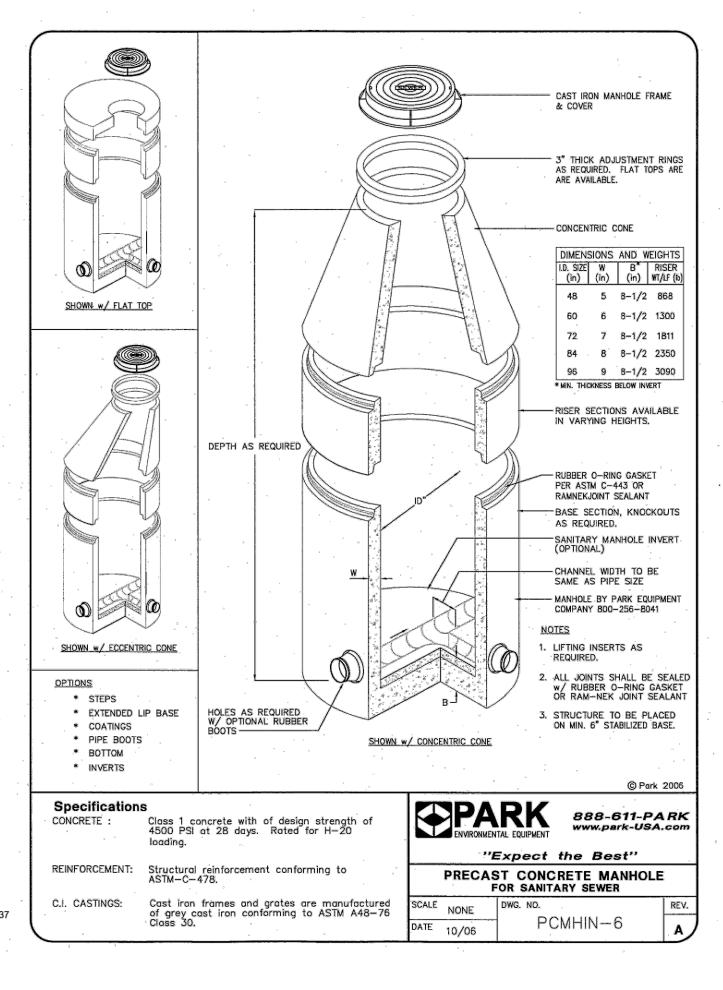
C9.1

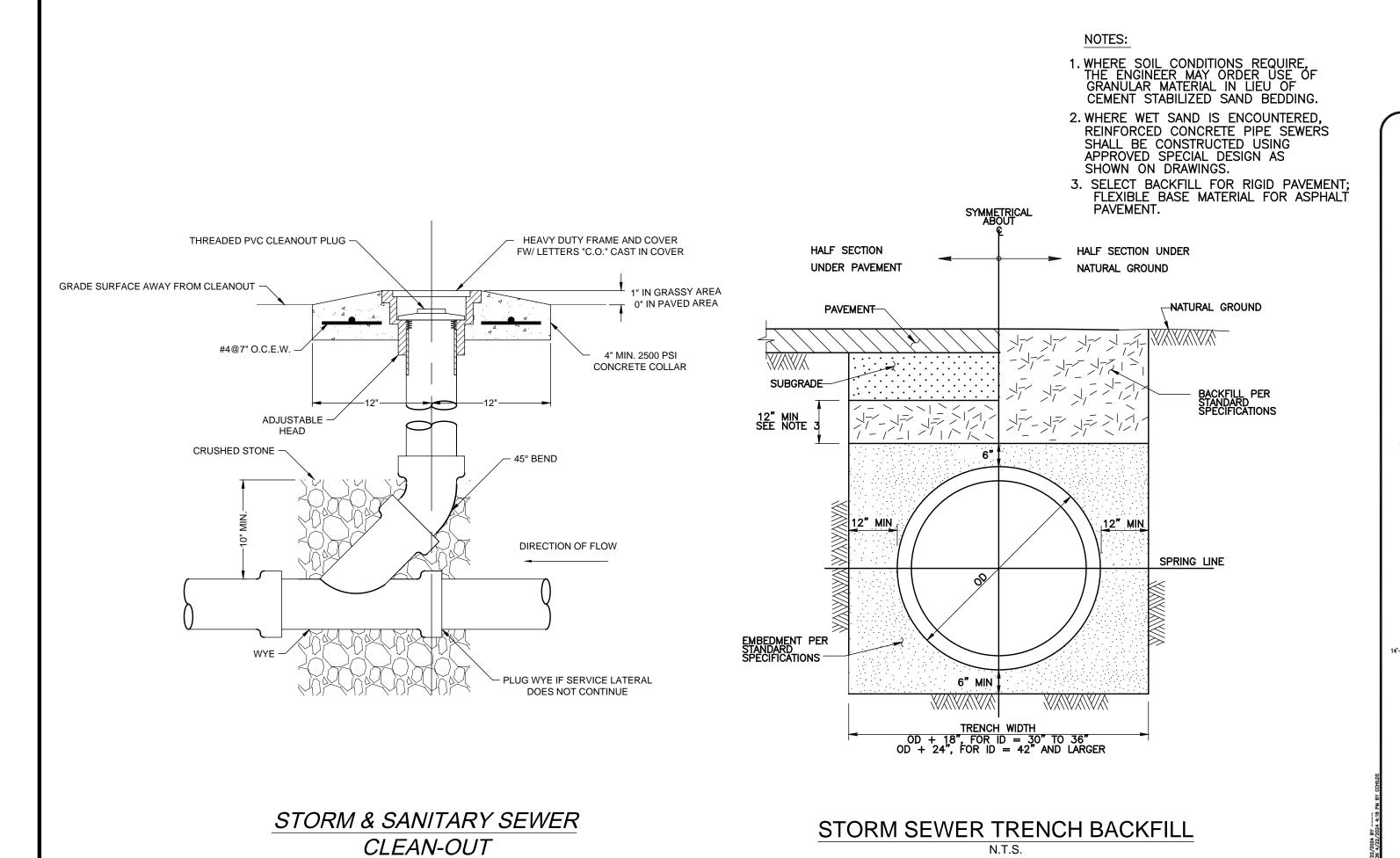
Drawing: Z:\00443 (Mark Dana)\007 Lost Oaks\CAD\C9.1 Water Details.dwg Last Plotted: Tue Apr 23, 2024 — 12:08pm By: ageldenhuys



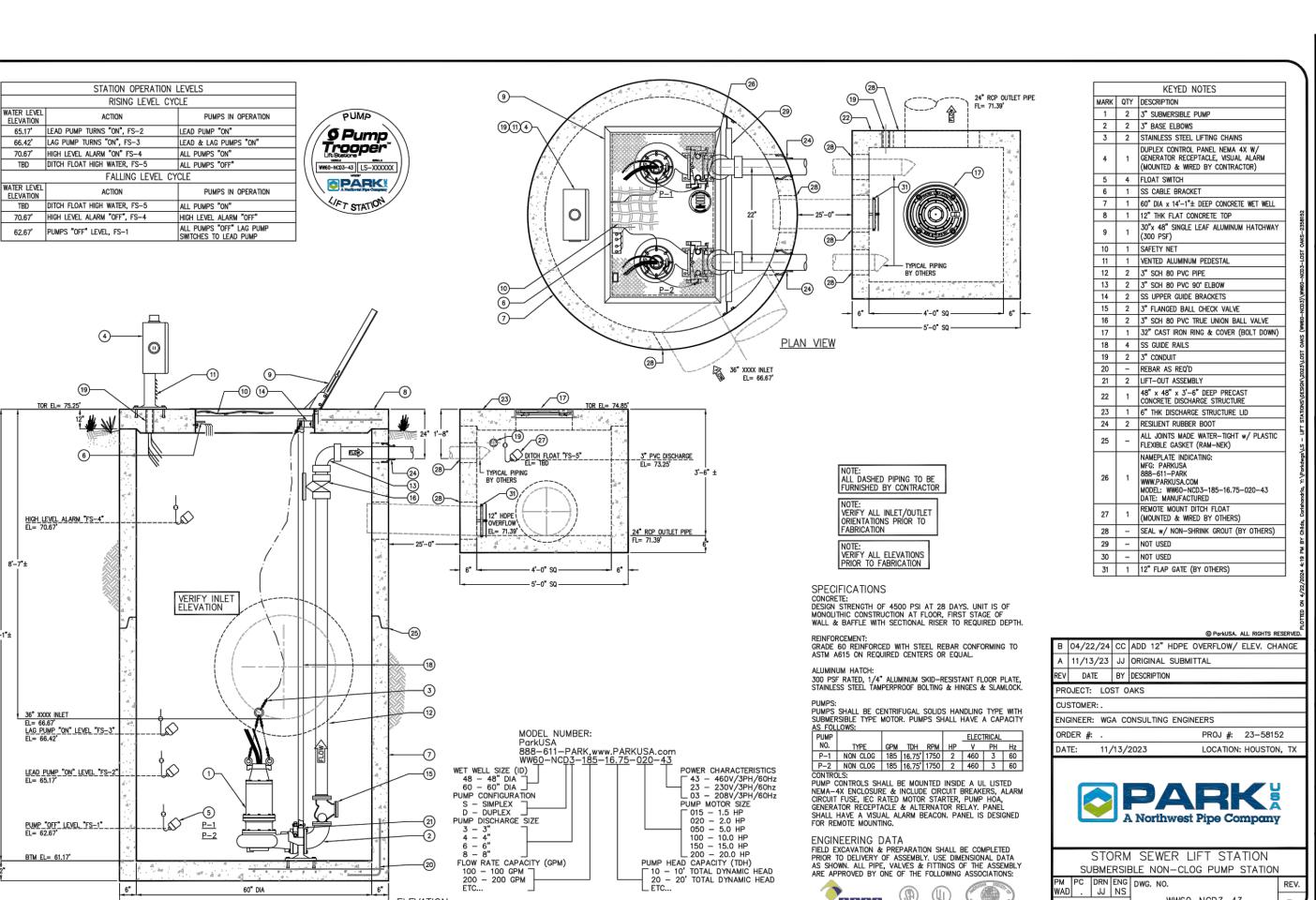








Drawing: Z:\00443 (Mark Dana)\007 Lost Oaks\CAD\C10.1 Storm and Sanitary Details.dwg Last Plotted: Tue Apr 23, 2024 - 12:46pm By. ageldenhuys



200 - 200 GPM

60" DIA

72" DIA



LOST OAKS

STORM SEWER AND

SANITARY SEWER

**DETAILS** 

AB, AG

SHEET

C10.1

. JJ NS 11/2023

WW60-NCD3-43

AB, AG